THELMA A BOLTIN CENTER / HISTORIC GAINESVILLE

516 NE 2ND AVE, GAINESVILLE, FL 32601

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Sheet List

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KATHLEEN KAUFFMAN

HISTORIC PRESERVATION OFFICER
DEPARTMENT OF SUSTAINABLE DEVELOPMENT
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ARCHITECT

REG ARCHITECTS, INC.

RICK GONZALEZ, AIA, PRESIDENT BRIAN LAURA, D. ARCH, SR. PROJECT MANAGER 120 SOUTH OLIVE AVE, STE#210, WEST PALM BEACH, FL 33401 PHONE: 561-659-2383

CIVIL ENGINEER

CHEN MOORE AND ASSOCIATES,

341 N. MAITLAND AVE.,SUITE 346 MAITLAND, FL 32751 +1(407) 536-7970 (OFFICE)

STRUCTURAL ENGINEER

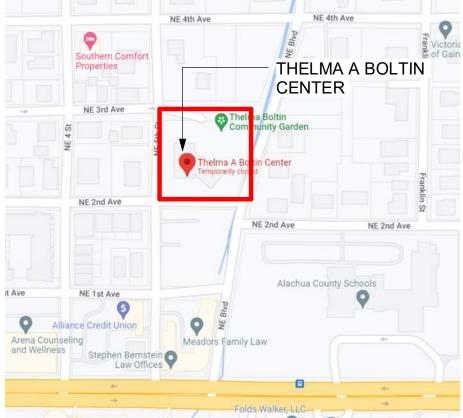
ONM & J STRUCTURAL ENGINEERS

JOE MINCUZZI, 1655 PALM BEACH LAKES BLVD., SUITE 204 WEST PALM BEACH, FL 33401

SCOPE OF WORK

EXISTING CONDITIONS PLANS /
CONCEPTUAL - ADA ACCESSIBILITY/
STRUCTURAL ANALYSIS / CIVIL ANALYSIS
HISTORIC PRESERVATION

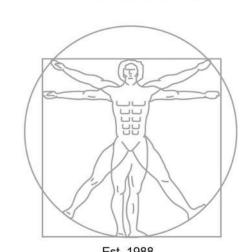
LOCATION MAP



MACRO - LOCATION



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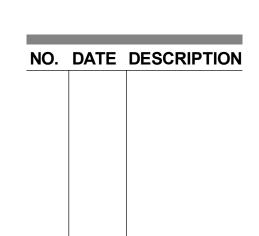


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1 of 26

SPECIAL THANKS TO THE STATE OF FLORIDA DEPARTMENT OF STATE HISTORIC RESOURCES GRANT & CITY OF GAINESVILLE, FLORIDA







VIEW FROM SOUTH ENTRANCE TOWARDS STAGE DURING A SERVICEMEN'S DANCE.



THELMA A BOLTIN SIGNAGE



VIEW FROM THE STAGE DURING A DANCE. EXISTING WOOD BEAMS VISIBLE ABOVE

HEIGHT OF CURRENT INSTALLED ACOUSTIC CEILING TILES.

SIGNAGE HISTORICAL PRESERVATION SIGNIFICANCE, 2008



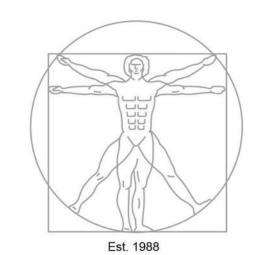
VIEW FROM THE SOUTH MAIN ENTRANCE

HISTORIC SIGNIFICANCE OF **GAINESVILLE HISTORIC /** THELMA A BOLTIN CENTER

The Thelma Boltin Center is an active City of Gainesville Parks, Recreations and Culture Affairs recreation amenity. It is a historic building located on a 2-acre site in the Gainesville North-East Historic District at 516 NE 2nd Ave. Originally built in 1942 to serve as a USO for service man and women from bases around Gainesville, the city later converted the building into a multipurpose facility. The Center now serves a wide variety of events, including dance groups, music recitals, concerts, family and social events, and political debates.



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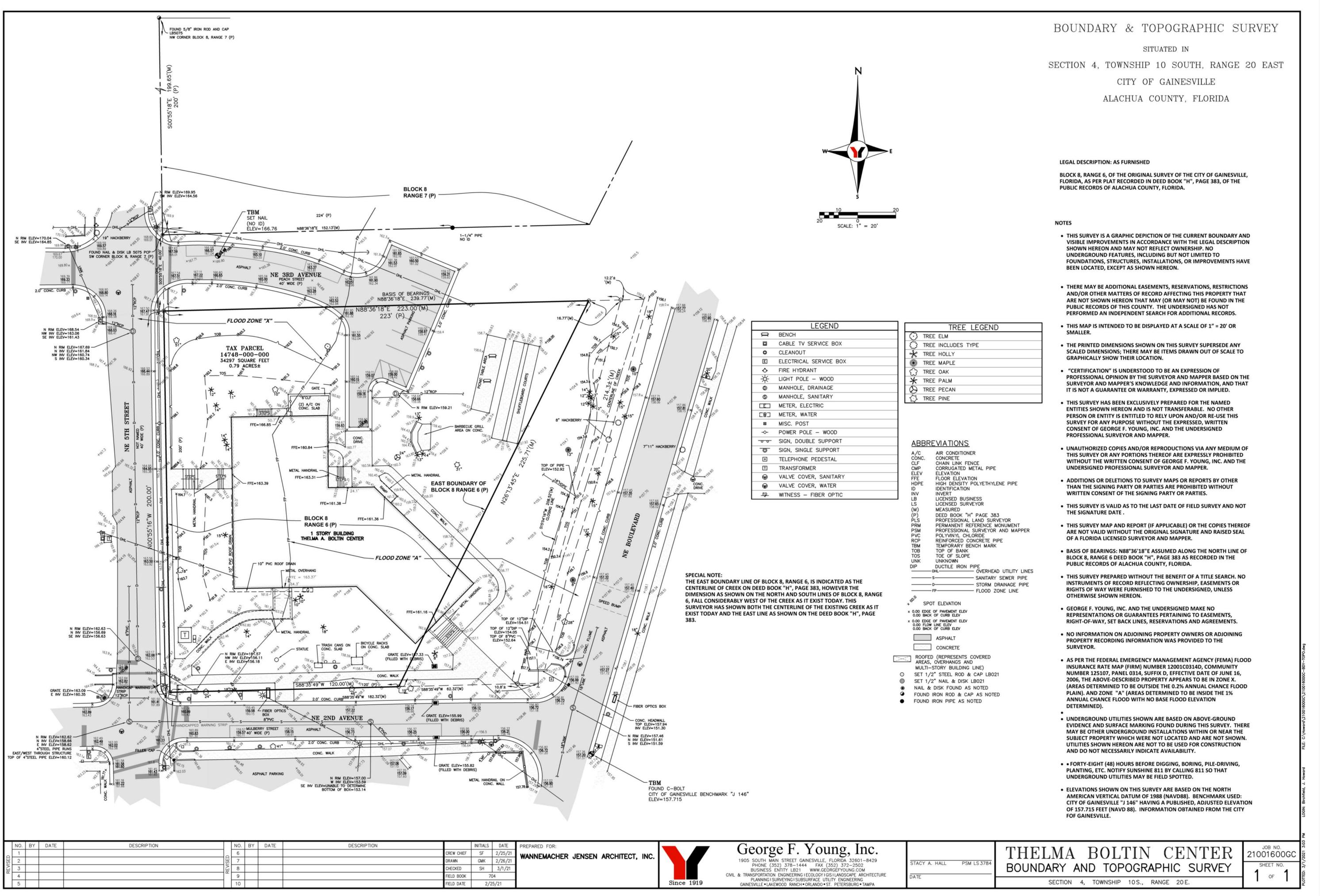
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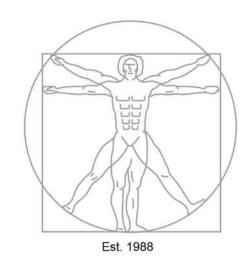
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HISTORICAL SIGNIFICANCE



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SURVEY



1 NORTH ENTRANCE



4 SOUTH (MAIN) ENTRANCE



7 EAST ENTRANCE



2 WEST ENTRANCE



5 SOUTH (MAIN) ENTRANCE



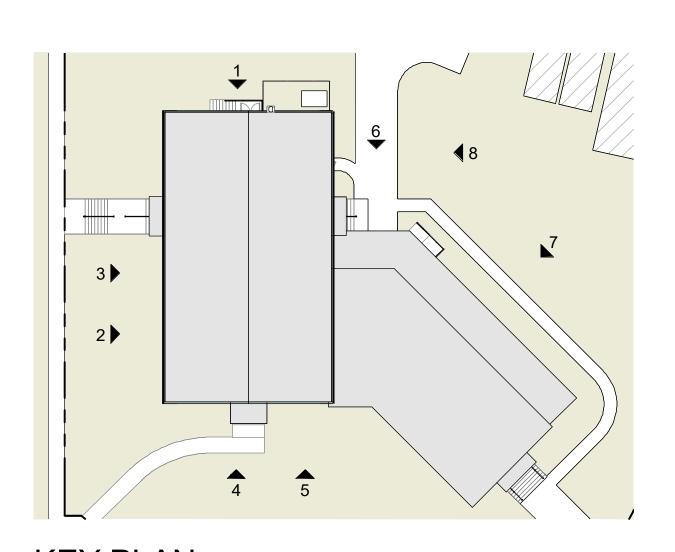
8 EAST ENTRANCE



3 WEST ENTRANCE

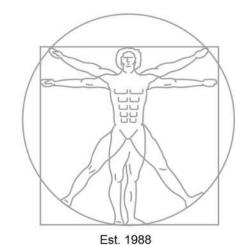


6 EAST FACADE ENTRANCE



KEY PLAN



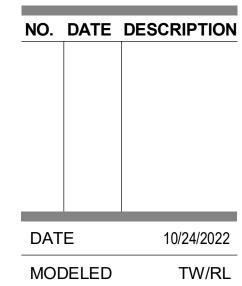


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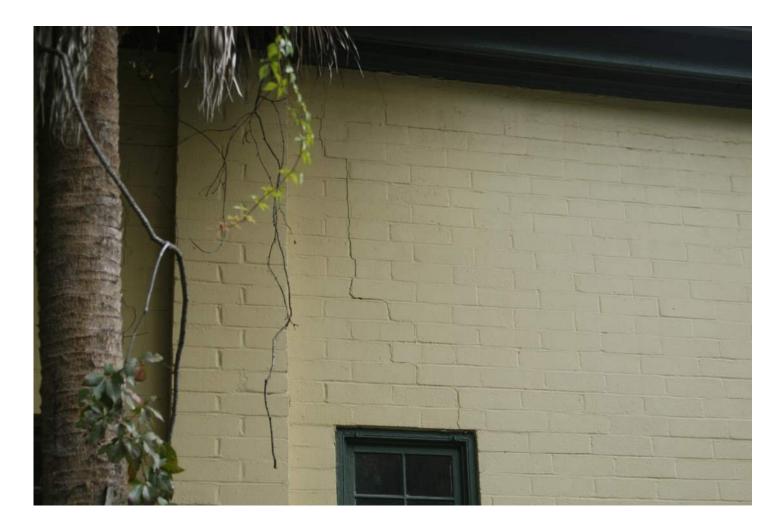
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EXTERIOR EXISTING CONDITIONS



1 CHIMNEY AT NORTH



4 CRACK THROUGH BRICK AT WEST



2 CHIMNEY AND ATTIC VENTS AT NORTH



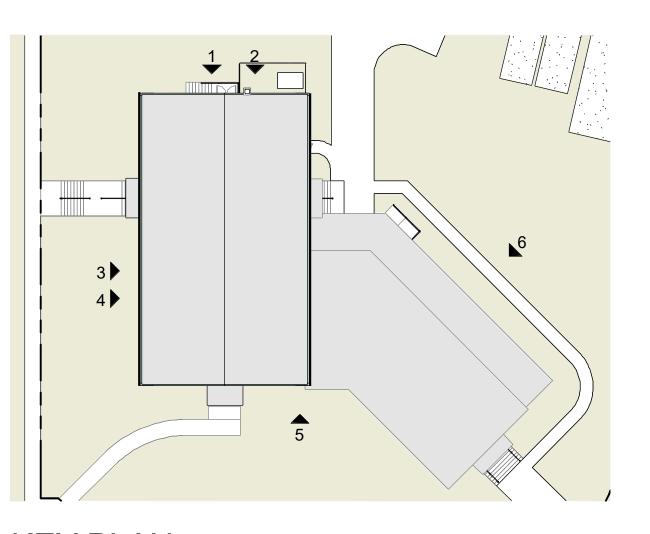
5 EAVE RETURN DETAIL AT SOUTH



3 GUTTER AND ROOF CONDITIONS AT WEST

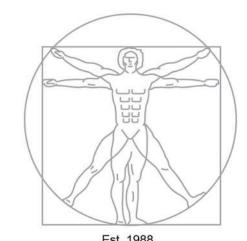


6 WALL PATCH AT EAST



KEY PLAN



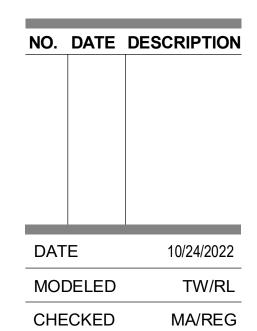


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EXTERIOR EXISTING CONDITIONS

AUDITORIUM DROP CEILING-

DOOR OPENING LEADS TO THE STAGE



DOOR LEADS TO THE GROUP ROOM

-WOOD FLOOR FINISH

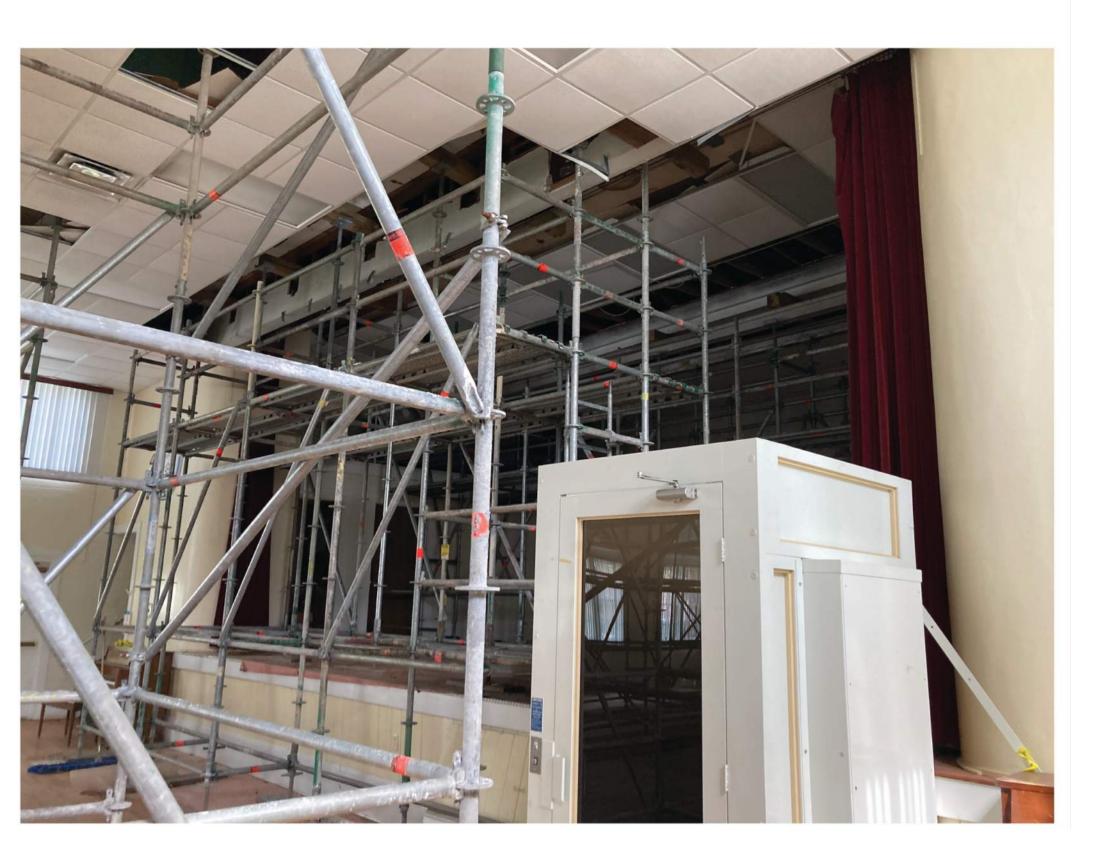


2 FIRST FLOOR AUDITORIUM

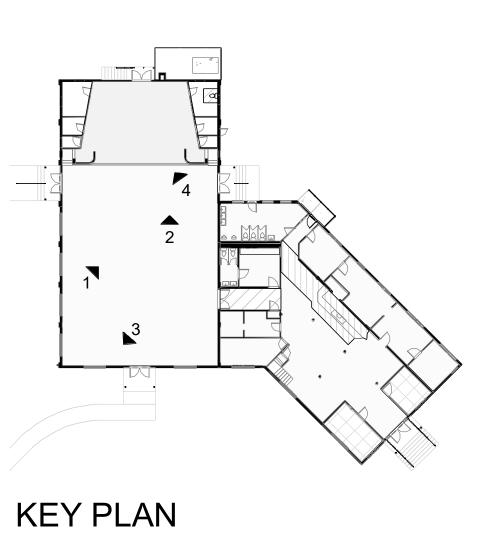




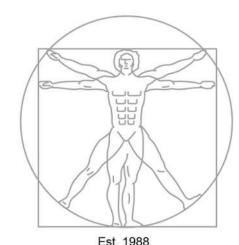
1 FIRST FLOOR AUDITORIUM



4 FIRST FLOOR, STAGE



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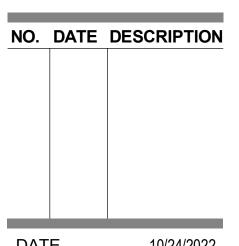


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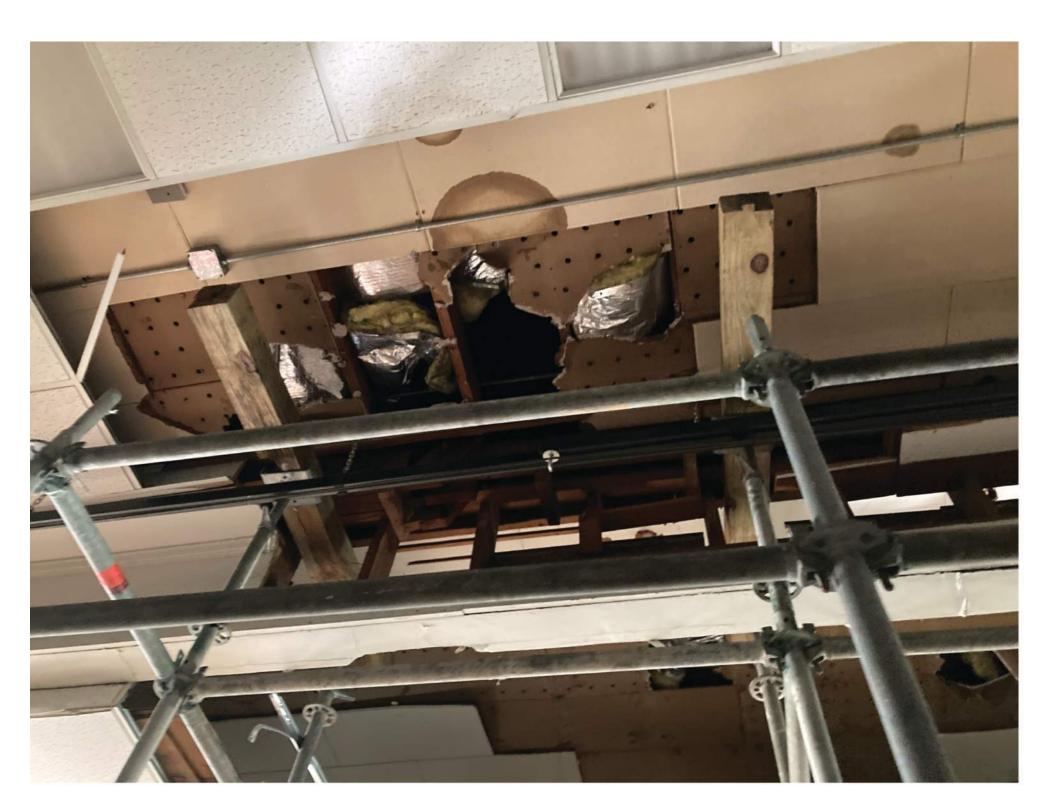


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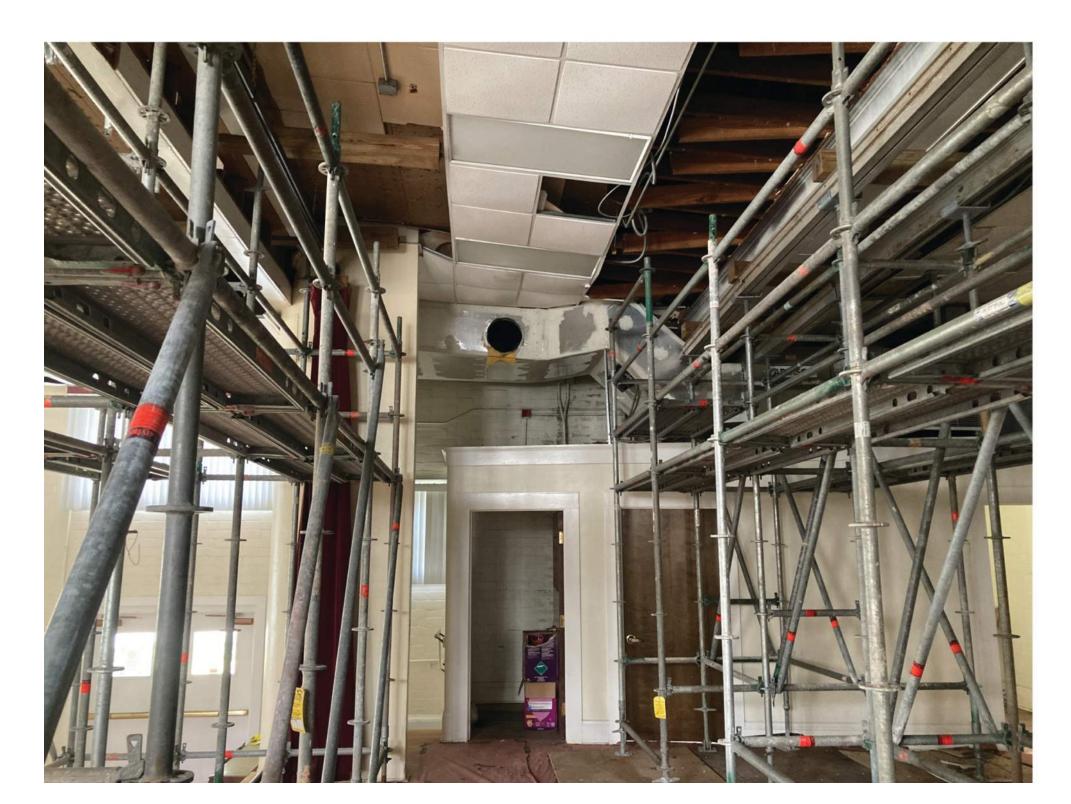
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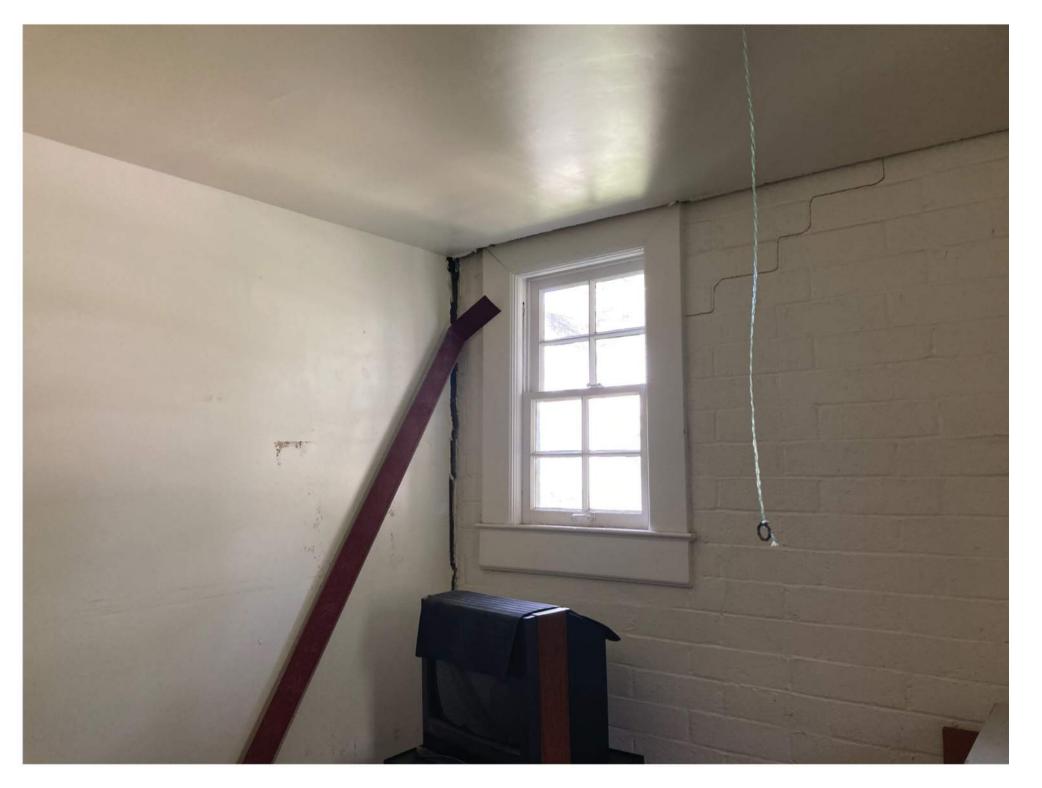
INTERIOR EXISTING CONDITIONS



1 CEILING ABOVE THE STAGE, FIRST FLOOR



3 STAGE, FIRST FLOOR

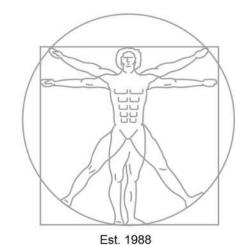


2 SUPPLY ROOM, FIRST FLOOR



4 STAGE CEILING, FIRST FLOOR



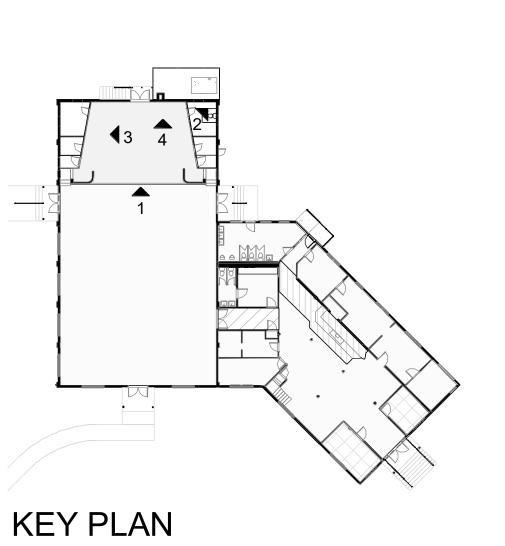


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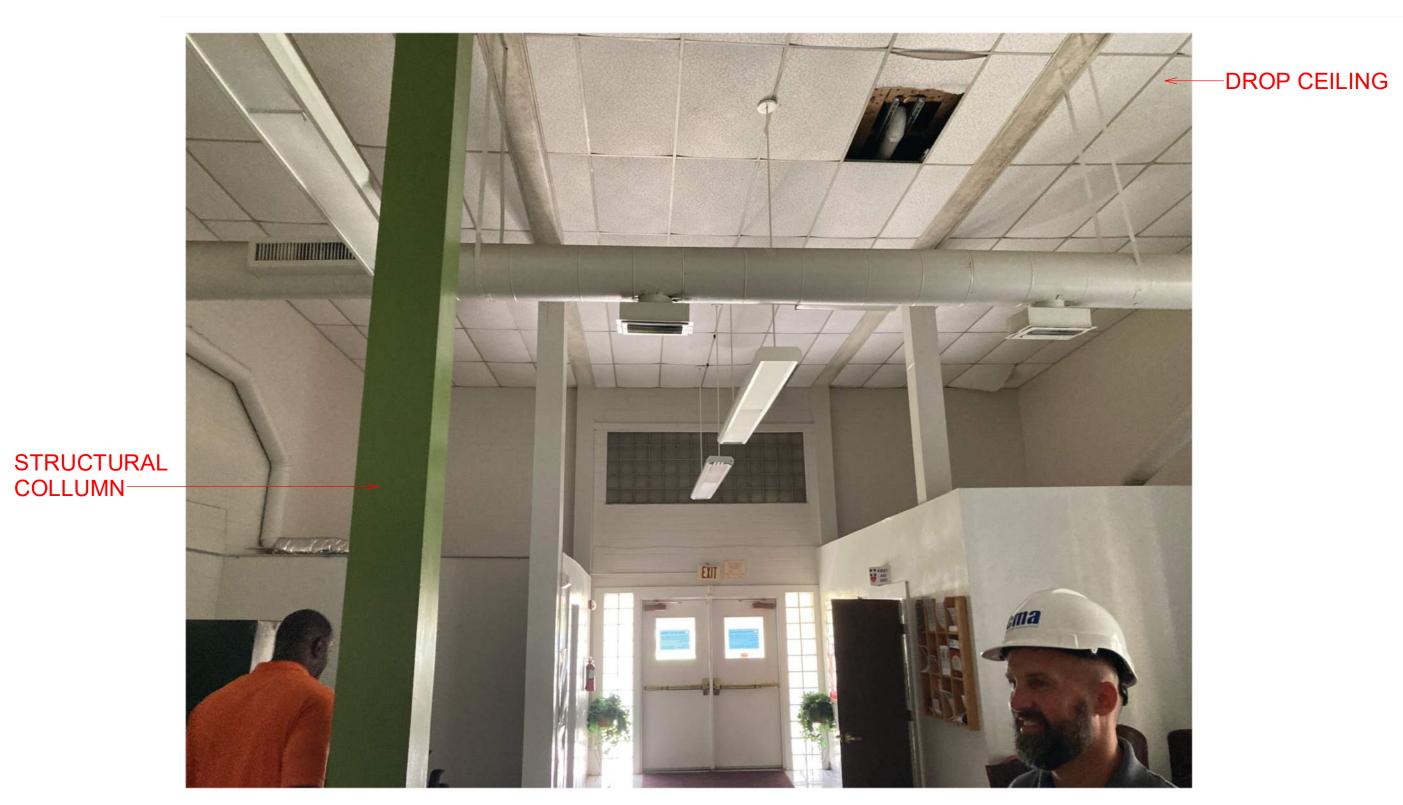
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INTERIOR EXISTING CONDITIONS



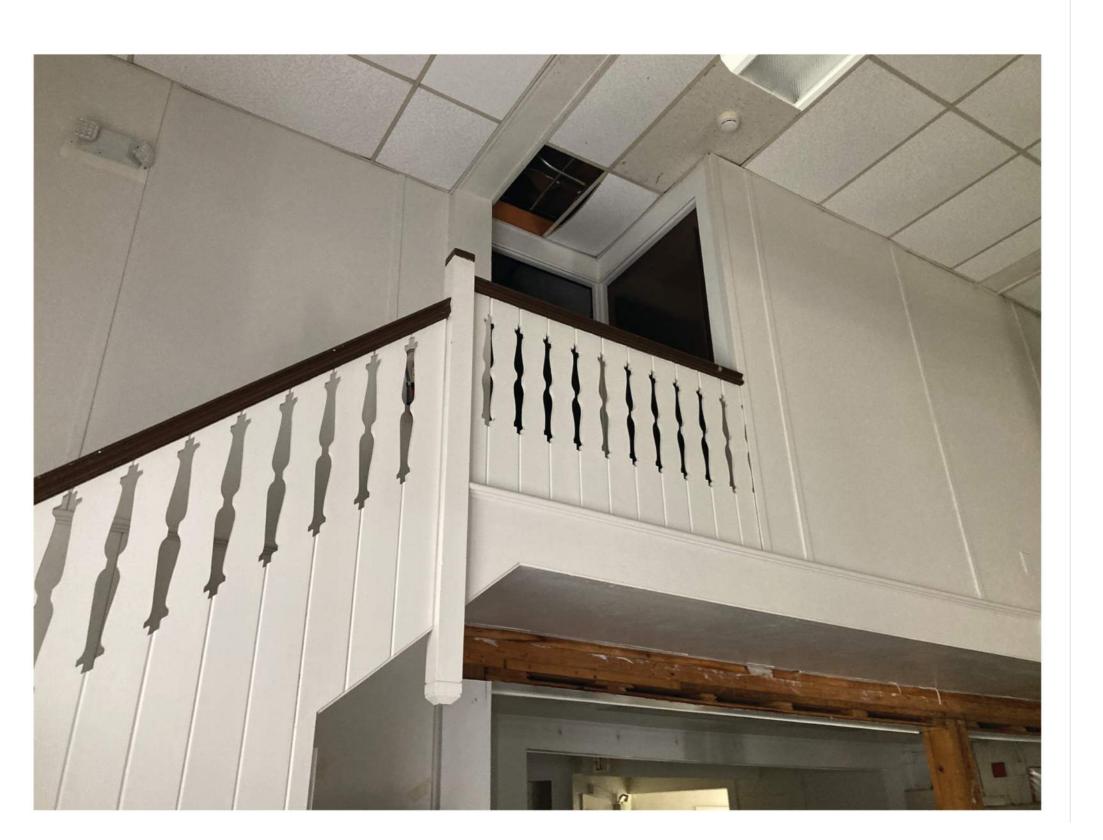
1 SOUTH ENTRANCE DOOR LEADS TO THE EXIT



3 FIRST FLOOR GROUP ROOM CEILING

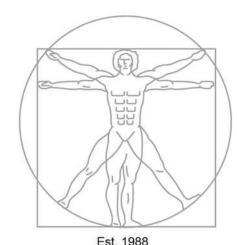


2 FIRST FLOOR GROUP ROOM



4 STAIRS LEAD TO THE SECOND FLOOR



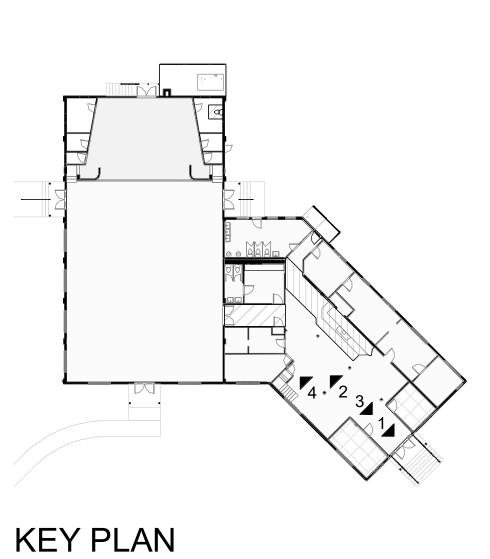


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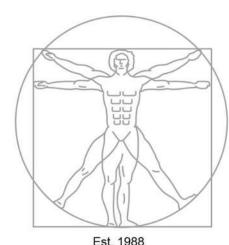
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INTERIOR EXISTING CONDITIONS





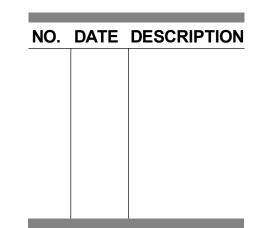


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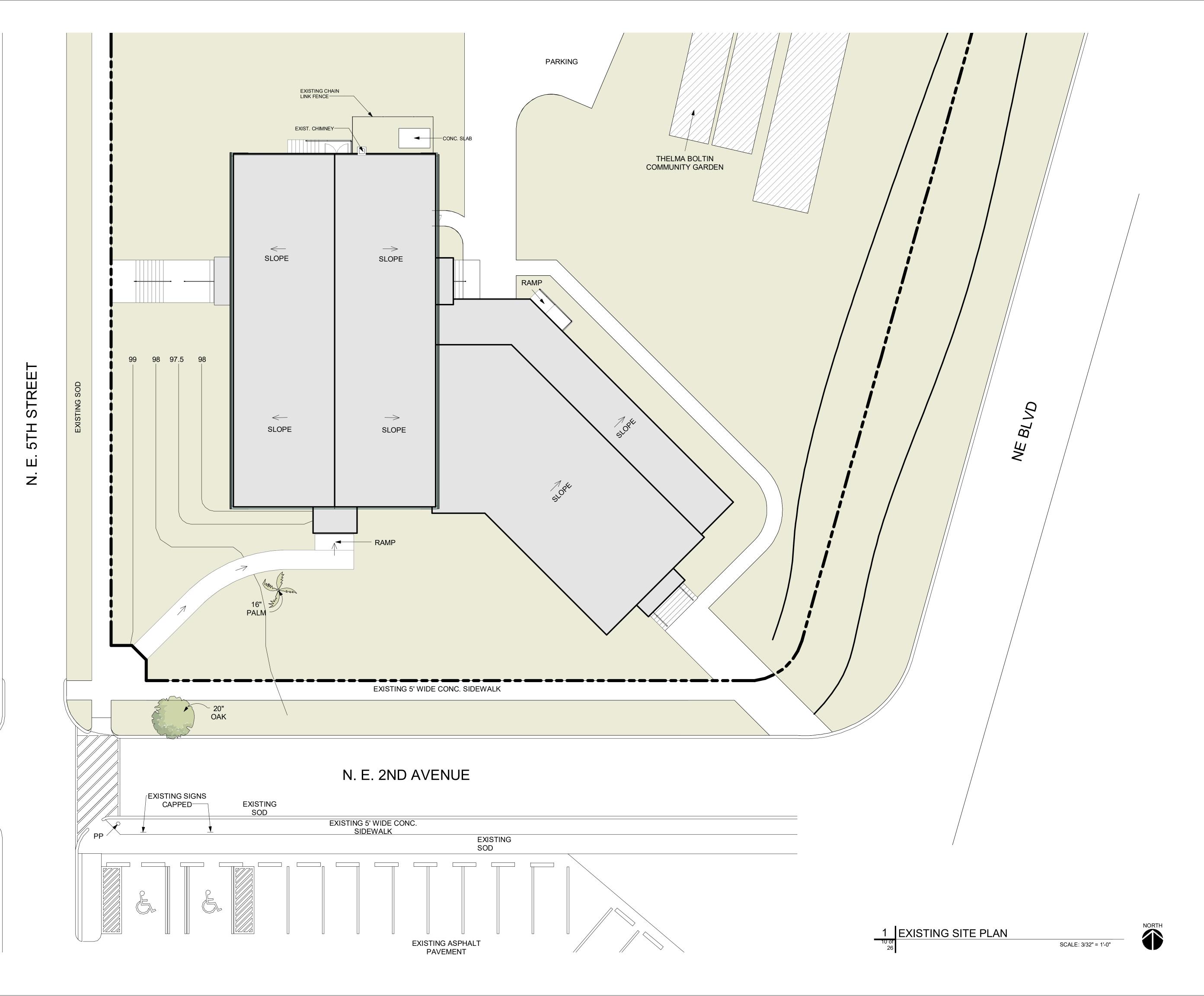
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PURPOSES ONLY; SEE SITE PLAN BY OTHERS.

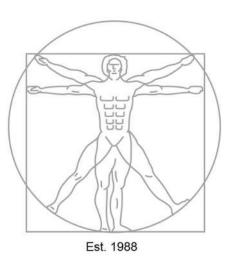
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EXISTING SITE AERIAL



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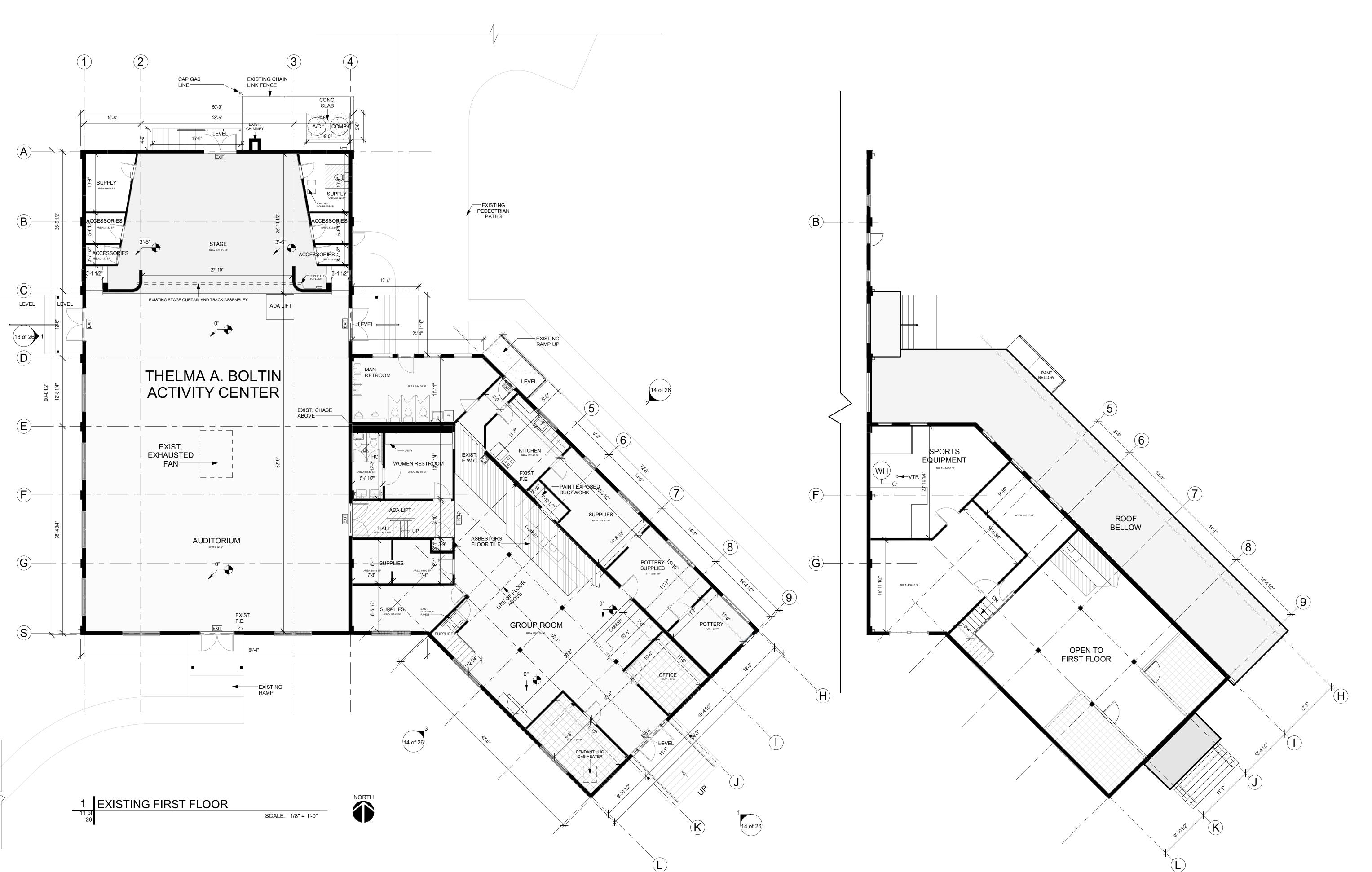
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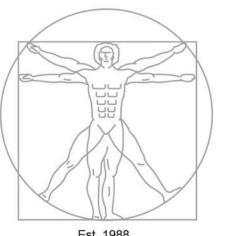
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EXISTING SITE PLAN







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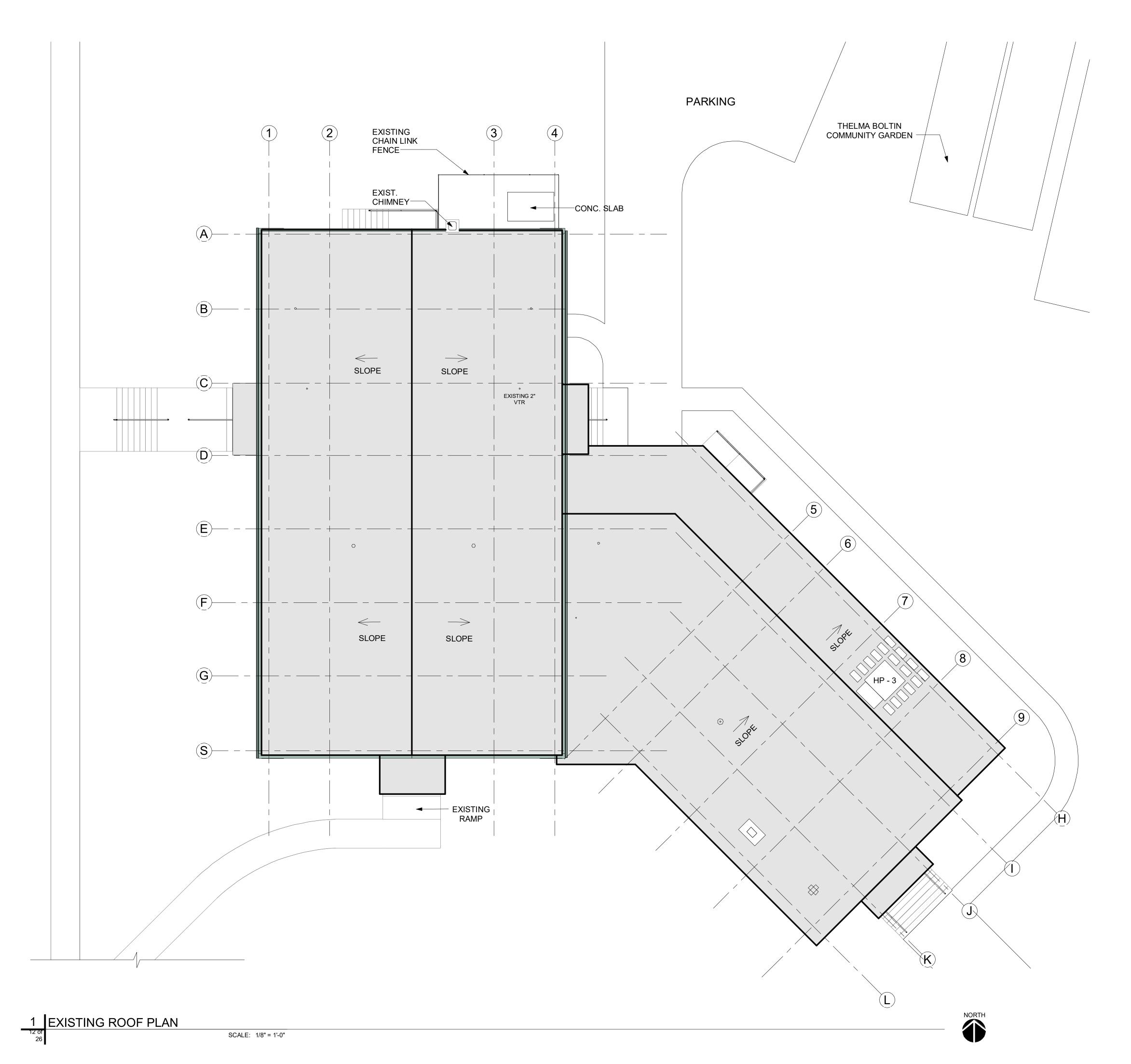
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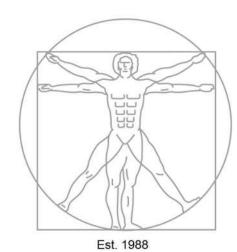
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EXISTING FLOOR PLANS

NOR





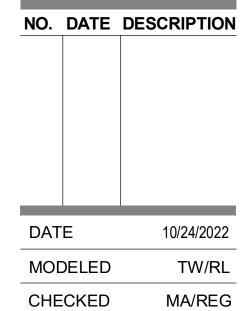


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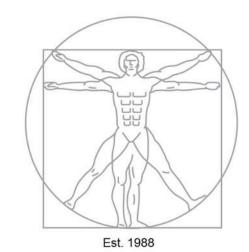
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EXISTING ROOF PLAN





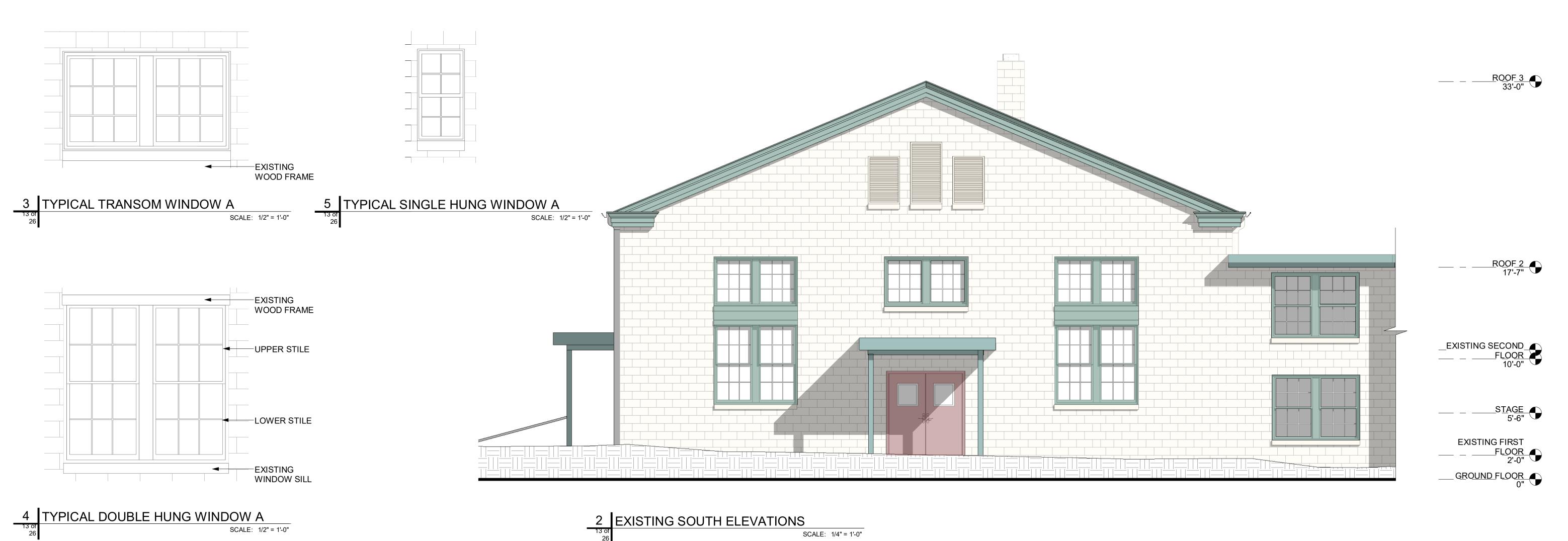


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EXISTING ELEVATIONS

2 EXISTING SOUTH ELEVATIONS SCALE: 1/4" = 1'-0"

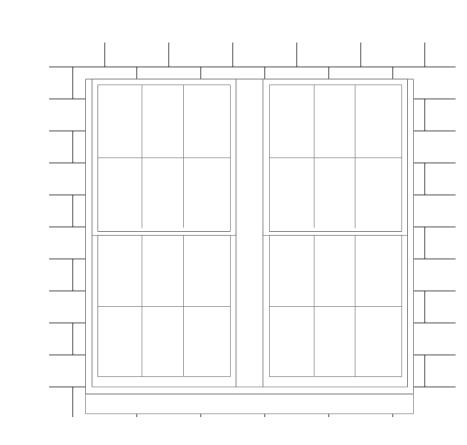
SCALE: 1/2" = 1'-0"



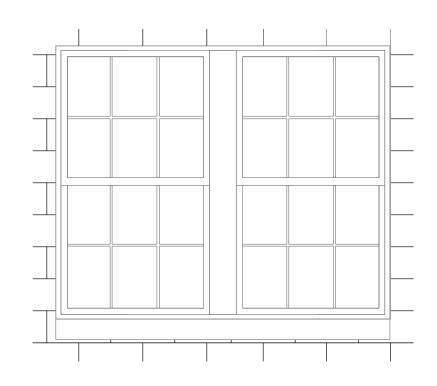
3 EXISTING SOUTH - WEST ELEVATION



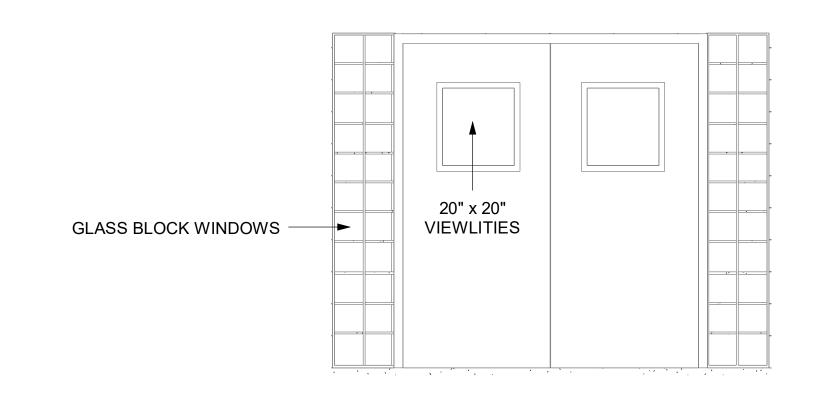




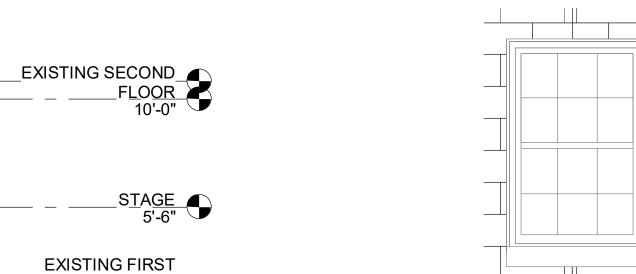
5 TYPICAL DOUBLE HUNG WINDOW B SCALE: 1/2" = 1'-0"



4 TYPICAL DOUBLE HUNG WINDOW C SCALE: 1/2" = 1'-0"



6 TYPICAL WOODEN DOUBLE DOORS SCALE: 1/2" = 1'-0"

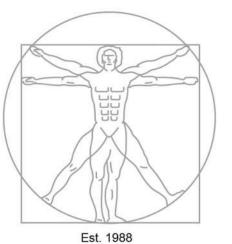


EXISTING FIRST FLOOR 2'-0" GROUND FLOOR
0"

ROOF 2 17'-7"

7 TYPICAL SINGLE HUNG WINDOW B SCALE: 1/2" = 1'-0"



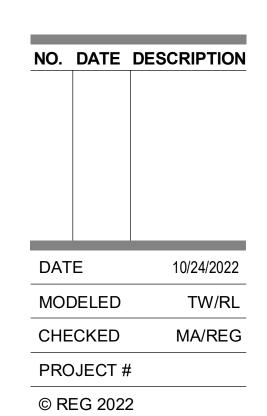


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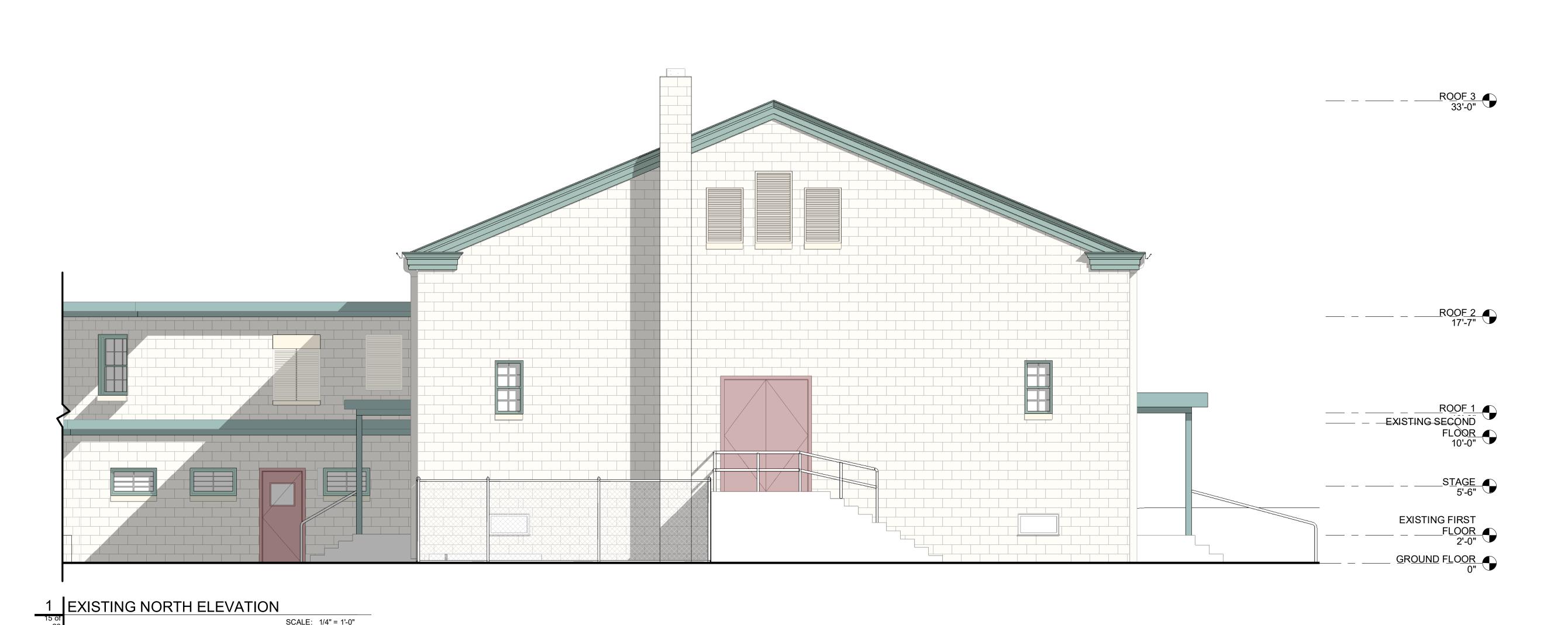
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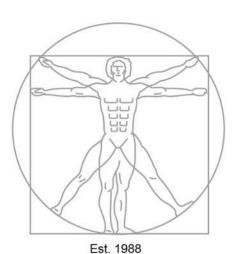
EXISTING ELEVATIONS





SCALE: 1/4" = 1'-0"





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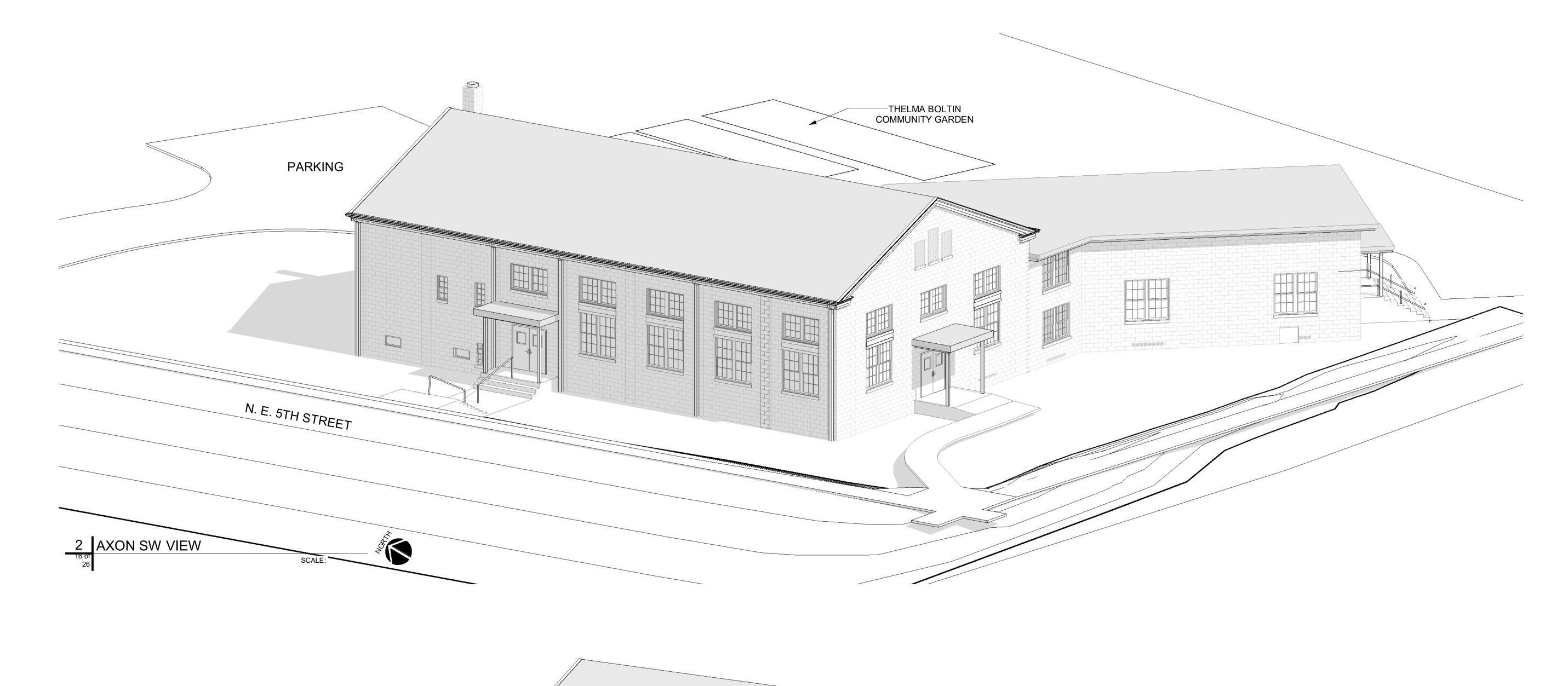
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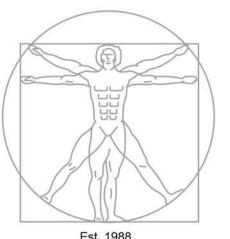
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EXISTING ELEVATIONS





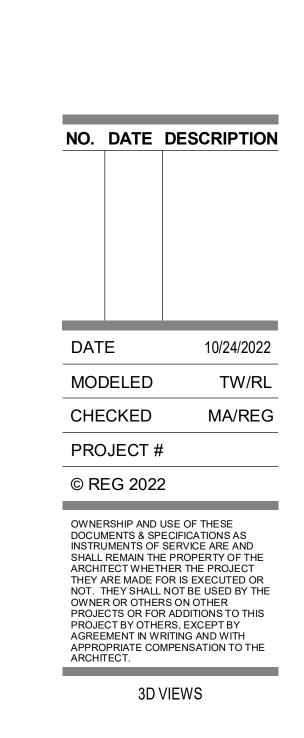


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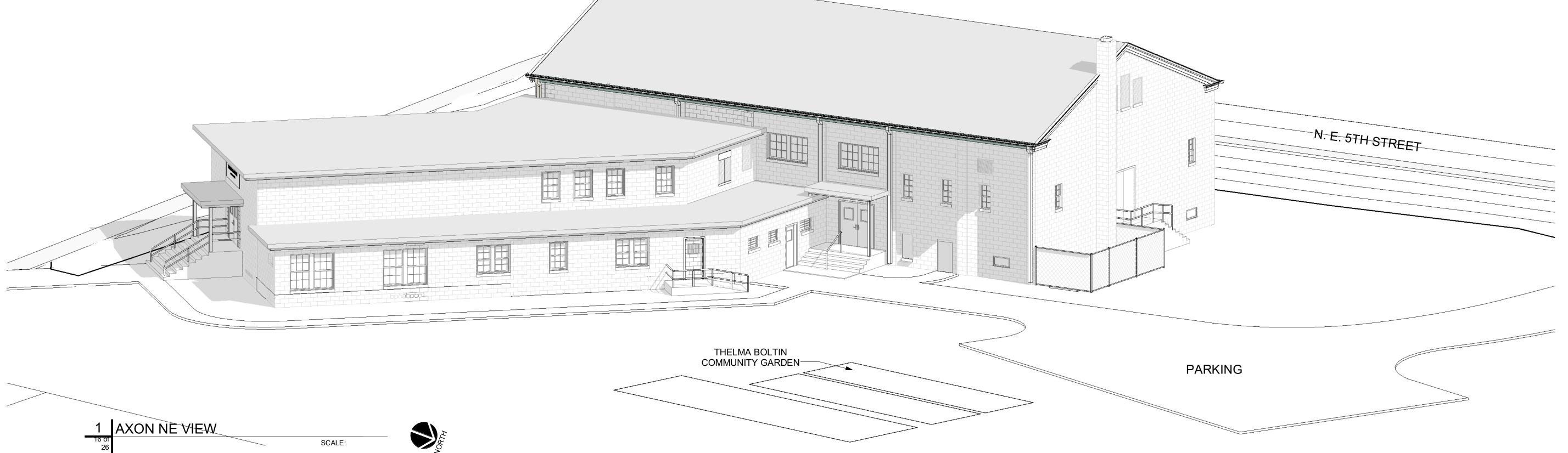
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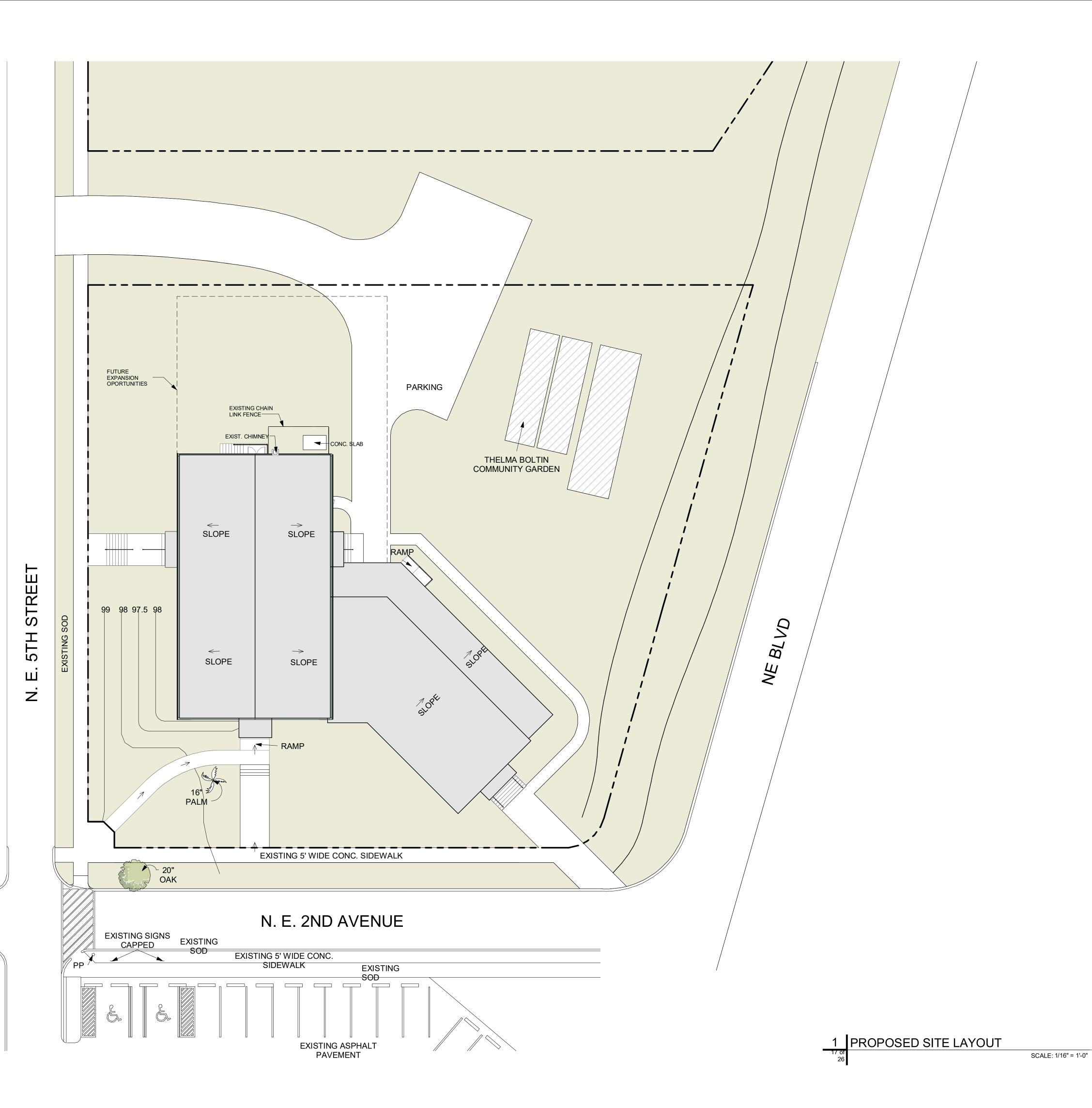
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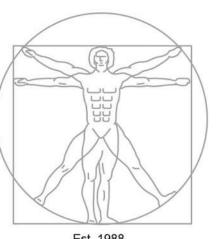










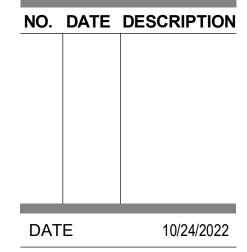


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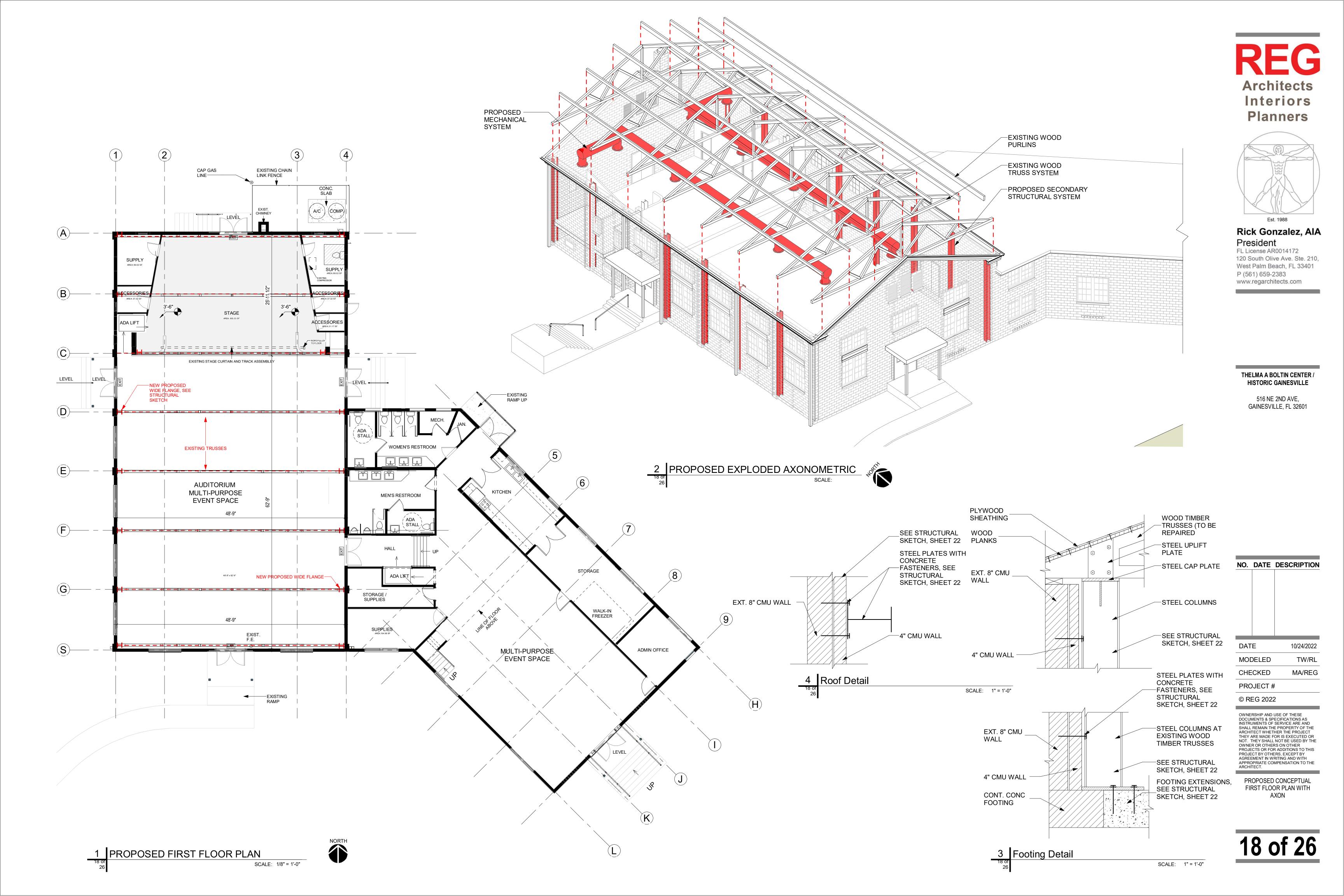
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PROPOSED SITE PLAN



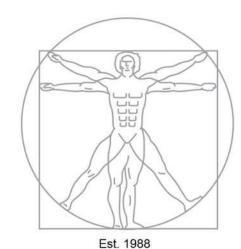


EXISTING WOOD PURLINS

EXISTING WOOD TRUSS SYSTEM

PROPOSED SECONDARY STRUCTURAL SYSTEM

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1 PROPOSED VIEW TOWARDS STAGE

19 of 26 SCALE:



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PROPOSED RENDERS



Memorandum

To: Brian Laura, D.Arch., REG Architects

From: Robert W. Best, Jr., P.E., Chen Moore and Associates

Date: October 10, 2022

Re: Thelma Boltin Center – Feasibility Study

CMA Project No.: 22-0219.00003

Chen Moore and Associates (CMA) has prepared this Memorandum to summarize the feasibility study findings pertaining to civil engineering for the above referenced project.

Backgroun

This project consists of an existing structure at 516 NE 2nd Avenue, Gainesville, FL 32601. The structure was built in 1942 to serve as a USO club catering to troops and was re-purposed after World War II as a community center. Walker Architects, Inc. prepared a Master Plan and Feasibility Study in 2019 which describes the history and condition of the facility in detail. The PRCA hired Wannemacher Jensen Architects in 2020 to prepare construction documents for a proposed \$2 million renovation of the facility with Wild Spaces & Public Places money. That was put on hold early in the process when structural failures were discovered.

The Gainesville City Commission has directed the Parks, Recreation and Cultural Affairs Department (PRCA) to move forward with the demolition and replacement of this historic building, but the City's Historic Preservation Board has asked that the PRCA explore every possible renovation/restoration option before proceeding with the building's destruction.

The Client has requested that the CMA Team (in collaboration with the REG Architects, Inc. (REG) and O'Donnell, Naccarato, Mignogna & Jackson, Inc. (ONM&J)) conduct architectural and engineering master planning services for a feasibility study of the Center, on behalf of the team.

CMA performed research and one site visit for this report. The research consisted of researching the FEMA FIRM Panels and Flood Zones. The site visit was performed on September 16, 2022 to assess the current conditions of the project site and the results can be seen in the following sections.

Flood Zone

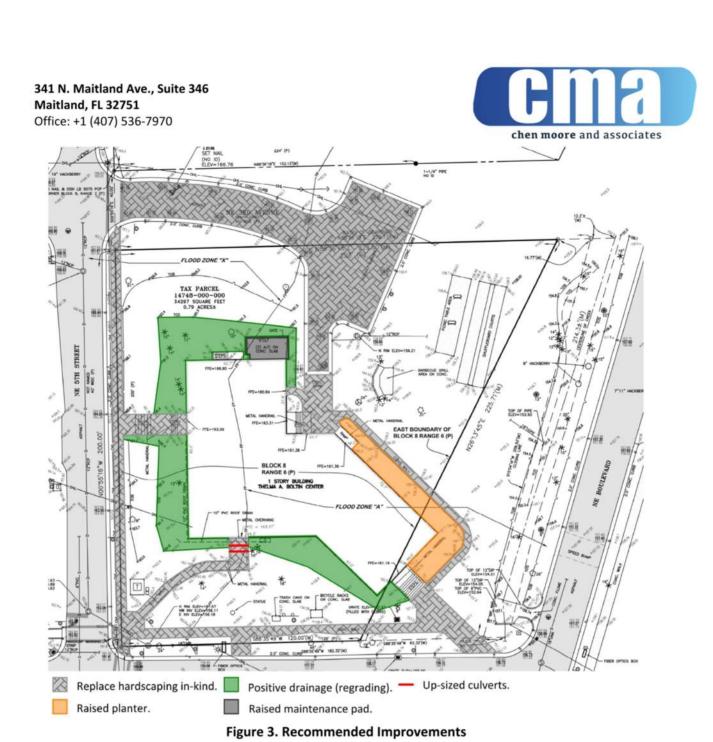
A portion of the project area lies within the Federal Emergency Management Association (FEMA) Flood Zone A per the Flood Insurance Rate Map (FIRM) Panels 12001C0314D. The attached FEMA FIRMette, Exhibit A, is included for reference. There is no identified Base Flood Elevation (BFE). During the site visit, City personnel noted that during regularly occurring rain events, the adjacent Sweetwater Branch Creek can rise and overflow its banks to the approximate limits shown in Figure 1.

Stormwater and Site Paving and Grading

This project area is within the Ocklawaha River Basin of the St. Johns Water Management District (SJRWMD). An environmental resource permit (ERP) from the SJRWMD is not anticipated for the scope of this feasibility study.

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Adjacent Roadway

The property is adjacent to City-owned roads (NE 2nd Avenue and NE 5th Street). Therefore, proposed site improvements shall be coordinated with the City of Gainesville Public Works. Should driveway improvements be proposed, an Obstruction of Right-of-Way with the City of Gainesville Public Works and a Driveway Permit with the City of Gainesville Department of Sustainable Development will be required.

Note, the civil scope excluded the following services: surveying, geotechnical investigation, traffic studies, environmental, off-site utilities improvements, off-site roadway improvements, building-related permitting, and cost estimates (in addition to the architectural cost estimates). The information found herein is preliminary and must be verified upon further design. Should you have any questions, please do not hesitate to contact me at my office at +1 (407) 536-7970, Ext. 1201, my cell phone at +1 (407) 506-4845 or send me an electronic message at rbest@chenmoore.com.

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341 N. Maitland Ave., Suite 346 Maitland, FL 32751

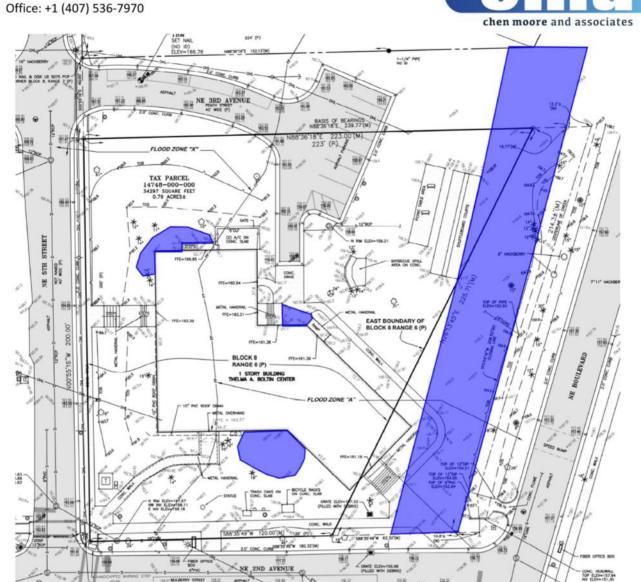


Figure 1. Existing Conditions

Area of known or suspected flooding/ponding/poor drainage.

The existing grading appears to transfer drainage around the building from the westside of the property to the eastside of the property. Based on the surveyed topographic elevations and the site visit, surface runoff does not flow eastward beyond the north side of the building other than potential sheet flow over the mechanical equipment maintenance pad located on the northside of the building (see Figure 1). Drainage heading eastward along the southside of the building eventually reaches the Sweetwater Branch Creek via rain leader piping, drains, and culverts (under hardscape landings). As the drainage flows eastward along the southside of the building, it flows directly against the foundation, and in some locations has been noted to pond. These are shown in Figure 1.

Due to unevenness in the sidewalks and concrete areas around the building, drainage ponding and even washing out of near the western access stairs has occurred (see Figure 2).

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Figure 2. Stairway Undermining

Proposed improvements to the site paving and grading are anticipated to consist of harmonization to connect the existing hardscaping, parking lot, and structure. The site grading shall consider elevation constraints, such as the adjacent roadways, public sidewalks, the existing onsite parking lot, historic flood levels, and the existing structure. Site grading shall promote positive drainage away from habitable spaces towards the stormwater inlets (catch basins). A minimum slope of 0.5% shall be maintained to create a dry swale around the perimeter of the building footprint and to minimize surface water ponding. The existing culverts under the southwest landing should be further investigated with regards to increasing their diameter. Due to historic flooding, a decorative retaining wall/planter shall be considered in the area adjacent to the southeast portion of the building. The hardscaping (walkways, parking lots, etc.) should be re-graded and replaced in-kind to eliminate ponding locations. The existing mechanical maintenance pad should be replaced with a further elevated pad to minimize drainage sheet flow across the maintenance pad. Maximum site slopes shall consider ADA criteria (typically 2% maximum slope) and slopes conducive to facility operations. These general improvements are shown in Figure 3.

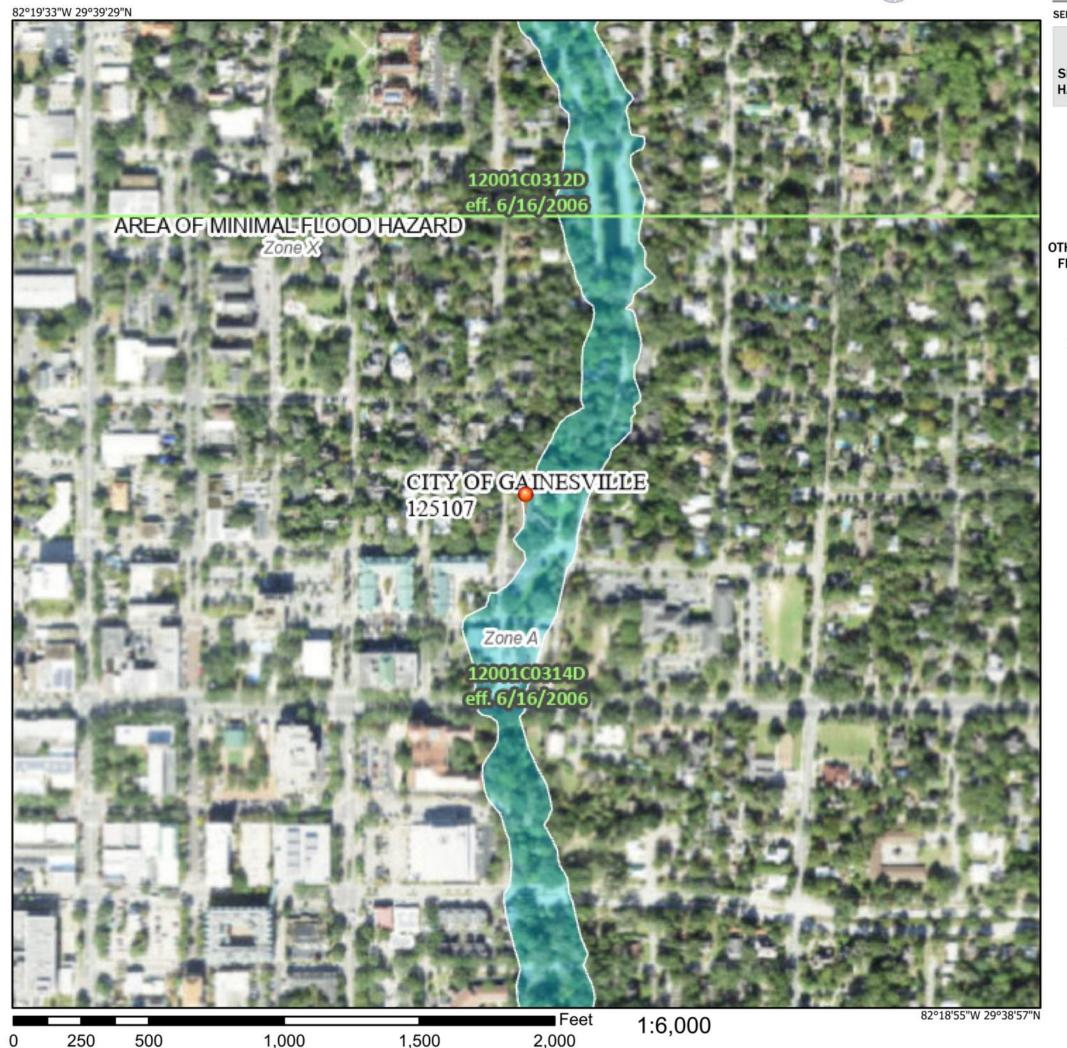
Water and Sanitary Sewer Utilities

This property is within the Gainesville Regional Utilities (GRU) service area for water and sanitary sewer. Proposed modifications to the water and sanitary sewer services are not anticipated, but should such improvements be proposed, those connections shall be coordinated with, and permitted through, GRU.

Page 3 of 4

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National Flood Hazard Layer FIRMette EXHIBIT A WE FEMA



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD Regulatory Floodway HAZARD AREAS 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D GENERAL - - - Channel, Culvert, or Storm Sewer STRUCTURES | LILLI Levee, Dike, or Floodwall (B) 20.2 Cross Sections with 1% Annual Chance 17.5 Water Surface Elevation 8 - - - Coastal Transect Base Flood Elevation Line (BFE) Limit of Study Jurisdiction Boundary --- Coastal Transect Baseline OTHER Profile Baseline FEATURES Hydrographic Feature **Digital Data Available** No Digital Data Available MAP PANELS The pin displayed on the map is an approximate

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

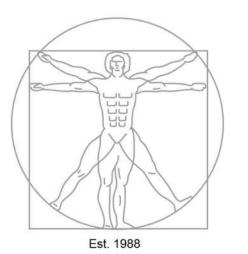
point selected by the user and does not represent

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 10/4/2022 at 5:11 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

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CIVIL ANALYSIS

Structural Engineers

O'Donnell, Naccarato, Mignogna Jackson, Inc.

Special Inspectors

October 20, 2022 **Brian Laura**

REG Architects, Inc. 300 Clematis Street, 3rd Floor

West Palm Beach, FL 33401

Thelma Boltin Center 516 NE 2nd Ave. Gainesville, FL 32601 Project No.: 341.179

SUBJECT: STRUCTURAL CONDITION ASSESSMENT

Pursuant to your request, we performed a site visit on September 16, 2022, at the above referenced project. The purpose of our site visit was to investigate the premises for signs of distress or deterioration and to determine if the building is salvageable.

This report is intended to present the results of our site visits. No destructive testing was performed. No existing drawings were available for our review. A report (Sputo and Lammrny Engineering, dated October 26, 2001) on the damaged roof was provided for our review. Our evaluation is based solely on our visual observations of the structural elements that were open and unobstructed on the date of our visit.

The 1-story, historic building consists of a roof / floor wood frame system supported by a single/double wythe, CMU exterior walls and interior columns.

Our observations revealed the following:

I. Exterior

A. North Elevation (Exhibit 1)

- 1. Exterior walls are painted CMU block. Block is in satisfactory condition except at northwest corner of the building. A crack is visible in the wall where the corner of the building has settled. It appears
- a repair was performed on the gable end in a continuous strip pattern located above the windows. 2. Wood framing around doors and windows are painted. The wood behind the paint has weathered over the years. Vertical cells of block are grouted in the header area of the door/ window openings and no other steel angle reinforcement is visible. (Exhibit 2)
- 3. Exterior walls of main hall are constructed of 8"x12"x4" CMU with two hollow cells with a second wythe of 4"x12"x4". The roof girders are supported on pilasters integral with all four walls of the main hall. (Exhibits 3 & 4)

B. East Elevation (Exhibit 5)

- 1. Exterior walls are painted CMU block. East elevation includes the main hall which connects to the adjacent building section at a 45-degree angle. A previous crack in the masonry wall has been patched over. (Exhibit 6)
- 2. Wood framing at doors and windows are painted. Previous weathering and some rot noticeable on the wood frames (Exhibit 7)

C. South Elevation (Exhibit 7)

- 1. Exterior walls are painted CMU and are in satisfactory condition. There are several cracks in the masonry walls that have been previously patched but no further cracking is visible. There is a low
- spot in the grade where water collects to the east of the hall entrance. (Exhibit 8) 2. Wood framing at doors and windows are painted. Paint is covering weathered wood. Entrance
- canopy is in satisfactory condition. 3. There are open vents to ventilate the crawl space under the ground wood floor. (Exhibit 9)

D. West Elevation (Exhibit 10)

- 1. Exterior walls are painted CMU. Block in is in satisfactory condition with the exception of the northwest corner of the building where some settlement of the foundation is visible. (Exhibit11)
- 2. Wood framing at doors and windows are painted. Paint is covering weathered wood. (Exhibit 12)

II. Interior

A. Roof

Roof at main hall is partially visible and access to roof is limited. Roof in main hall is currently shored. Two girder trusses at stage has failed and is fully shored. Some termite damaged, wood was also observed. (Exhibit 13)

1. The wood floor appears to be level and solid. They are supported by a wood beam/ joist frame system supported by 12" x 12" CMU piers. Flooring that is visible appears to be in satisfactory condition. (Exhibit 14)

C. Columns and beams

Interior columns support the roof structure at the side building. One of the columns was exposed. The exposed column was constructed of vertical boards fastened together. The wood on the exposed column appeared to be in good condition. The composition and construction of the other columns could not be determined as they were covered with trim material. (Exhibit 15)

III. Conclusion

A. Roof

1. The two shored girder wood trusses over the stage area will require replacement. The remaining girder wood trusses can be repaired (e.g double wood scabbing, additional metal plate connections, etc.). The extent of repair will depend on the as-built condition of the wood. A design analysis will also reveal if the girder trusses/ purlins will require additional reinforcement to support loads per the current code. Additional column supports/ footings at the girder truss bearing ends will be required to properly transmit all loads to the ground per the current code. These new columns can also provide some lateral bracing to the existing exterior walls. New plywood sheathing above the existing wood planks (to be fasten into the roof framing members) will be required to provide an adequate roof diaphragm to resist lateral loads per the current code. Mechanical uplift connectors will be required throughout the roof framing which were found missing.

Ceiling wood joists will also have to be repaired/ replaced as required.

B. Exterior Masonry Walls

- 1. The exterior step cracks are indicative of different settlement and subsurface consolidation beneath the foundation in the affected area. The total extent of the subsurface consolidation and settlement is a function of the soil conditions that exists and applied loads. Thus, we cannot determine whether the settlement has subsided or will worsen without knowledge of the underlying soil. A subsurface soil investigation will be required which includes borings and/ or hand augers located near the affected areas.
- 2. Further field investigation required to determine the sizes of the existing footings so that we can determine the adequacy of the existing foundation system.
- 3. The door/ window headers will require reinforcement with either a new precast / cast -in place lintel or a new steel angle to support the required gravity loads.

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- 4. The exterior walls/gable end will require additional lateral reinforcement due to their unbraced height to resist the required wind loads. We also question whether the door / window jambs are sufficiently reinforced to resist the required wind loads.
- 5. In addition, all pipe penetrations should be sealed to mitigate water intrusion. The exterior should be caulked and painted with a waterproof coating.

C. Building Envelope

1. It is evident that the existing door / window openings are not impact rated.

The building overall was found in satisfactory condition but will require the above repairs. Two wood girder trusses over the stage area will require replacement and further investigation required to determine the extent of damage on the remaining wood girders. Further investigation required to determine the settlement issue at the NW corner. If the building is to remain, the Florida Existing Building Code may mandate that the entire building be upgraded to current code due to the number of repairs. This upgrade will not only include the structure, but also all other building systems (mechanical, electrical, plumbing, life safety, accessibility, etc.). We highly recommend that the building official be consultant to determine the extent upgrade required for a Historic Building. The structure may be salvageable but the structural / architectural upgrades to retrofit this existing building may be cost prohibitive.

IV. GENERAL NOTES

We were commissioned by REG Architects to perform a general walk-through, visual review of the building, advising them of any structural defects and need for repair. This report is based upon clearly visible, open, and unobstructed areas of the premises on the date of our site visit. No opinion is rendered with regards to structural elements that were concealed.

Jeff Ballard

Project Engineer

We trust that you will find the information contained herein helpful. Should you have any questions, do not hesitate to call our office.

O'DONNELL, NACCARATO, MIGNOGNA & JACKSON, INC.

Joseph F. Mincuzzi, P.E. Vice President STATE OF FLORIDA Registered Professional Engineer No.: 38162 Registered Professional Special Inspector No.:0952

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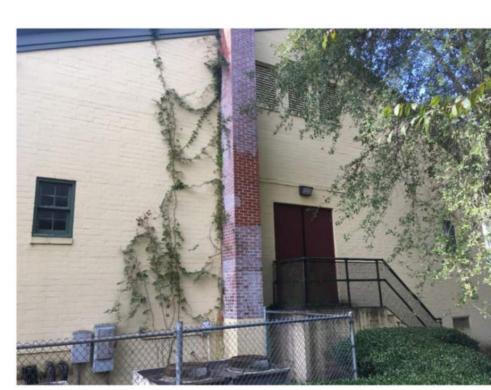




Exhibit 2



Exhibit 4



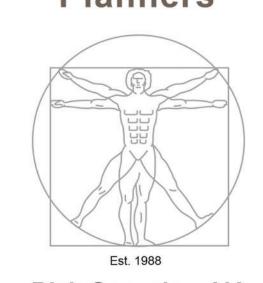


Exhibit 6





Exhibit 8



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Exhibit 13



Exhibit 14



Exhibit 9



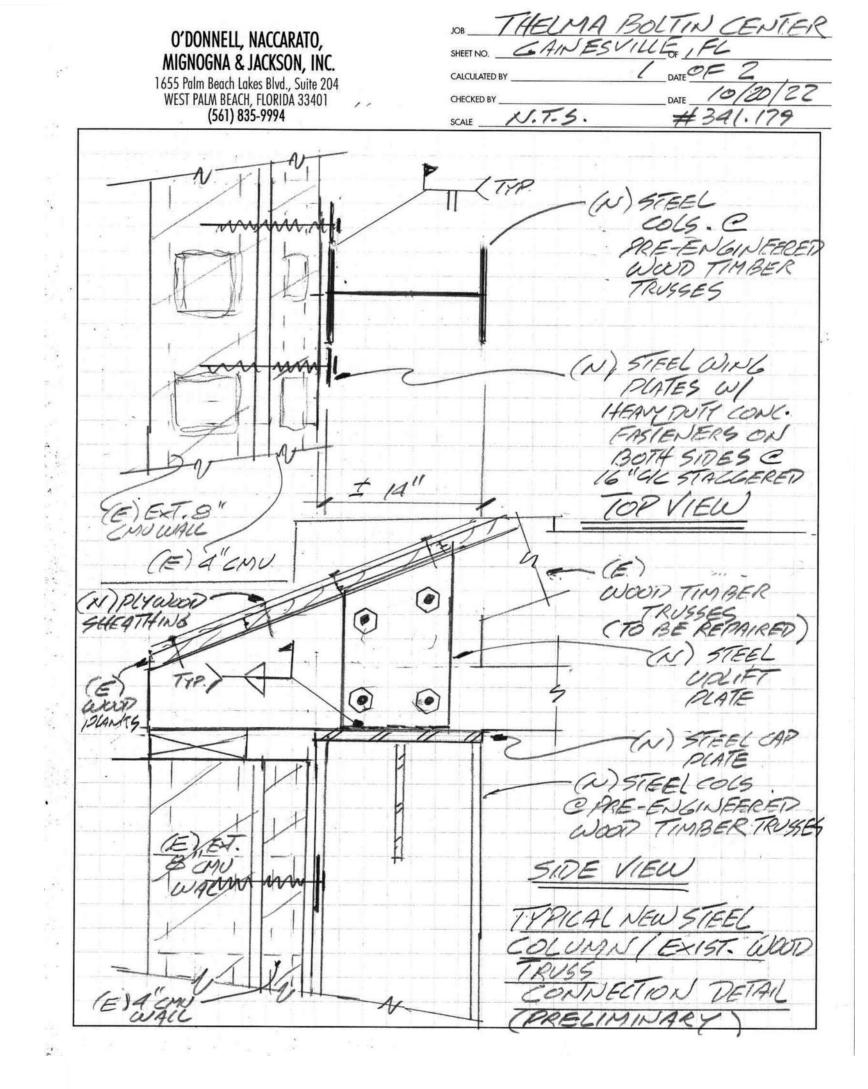
Exhibit 10

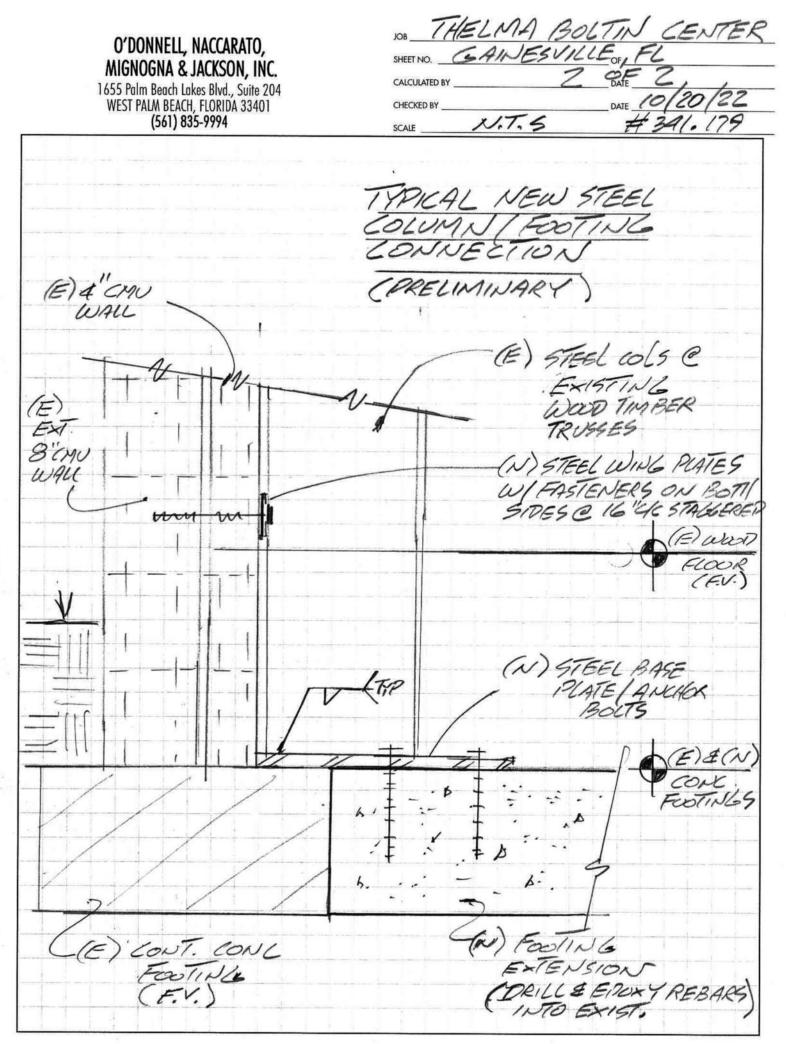


Exhibit 15

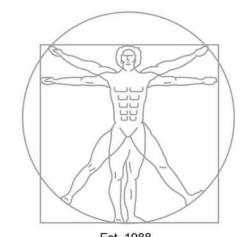


Exhibit 12







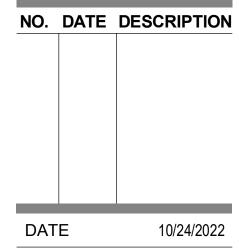


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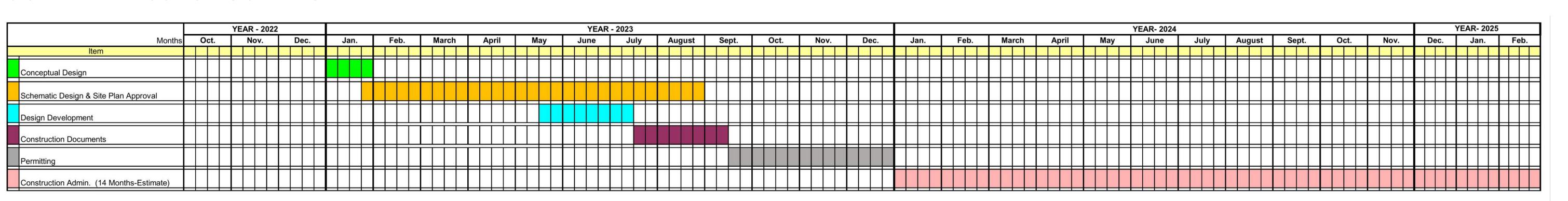
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STRUCTURAL CONDITION ASSESSMENT

OUTLINE PROJECT SCHEDULE



NOTE A: Project schedule is subject to change dependent on Owner and Contractor input as well as the City of Gainesville Site Plan Approval and Permitting Process.

NOTE B: Schedule assumes 1-round of comments, additional round(s) of comments will increase schedule.

NOTE C: Schedule does not include additional reviews by building department, engineering, etc.

NOTE D: Schedule assumes replatting the site is not required.

The following includes a low, medium, and high estimate of cost for the overall site and proposed building with additional reports and renovation/restoration costs to be considered when pursuing grants and other financing options.

ESTIMATED PROJECT COST / BUDGET

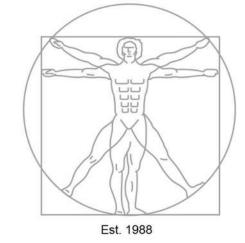
*ALL ESTIMATES ARE SUBJECT TO CHANGE AS PER MARKET CONDITIONS.

Thelma A Boltin Budget Building Historic Renovation	SF	Low \$/SF 350.00	Medium \$/SF 400.00	High \$/SF 450.00
SUBTOTAL	10,543 SF	3,690,050	4,217,200	4,744,350
Site Improvements (Allowance) OH Profit 18% Soft Cost 10% (A&E plans) TOTAL		65,000 664,209 369,005 4,788,264	70,000 759,096 421,720 5,468,016	75,000 853,983 474,435 6,147,768
ADDITIONAL REPORTS Structural Environmental Level 1/Paint Ar Moisture/Thermal Survey M/E/P Evaluation	9,500 9,500 4,500 2,500	10,000 10,000 5,000 3,000	12,500 12,500 5,000 4,000	
REPORT TOTAL		26,000	28,000	34,000
GRAND TOTAL		4,814,264	5,496,016	6,181,768

EXCEPTIONS

Major Unforseen Structural Repair Archeological Report/Approval



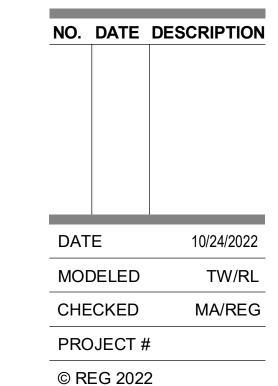


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THELMA A BOLTIN CENTER / HISTORIC GAINESVILLE

516 NE 2ND AVE, GAINESVILLE, FL 32601



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ESTIMATED PROJECT COST & SCHEDULE

SECOND FLOOR ALTER AND SEATING AREA BEHIND MAIN STAGE

CASE STUDY 01

OLD MT. CARMEL CHURCH GAINESVILLE, FLORIDA



Size: 12,561 SF / \$446 SF

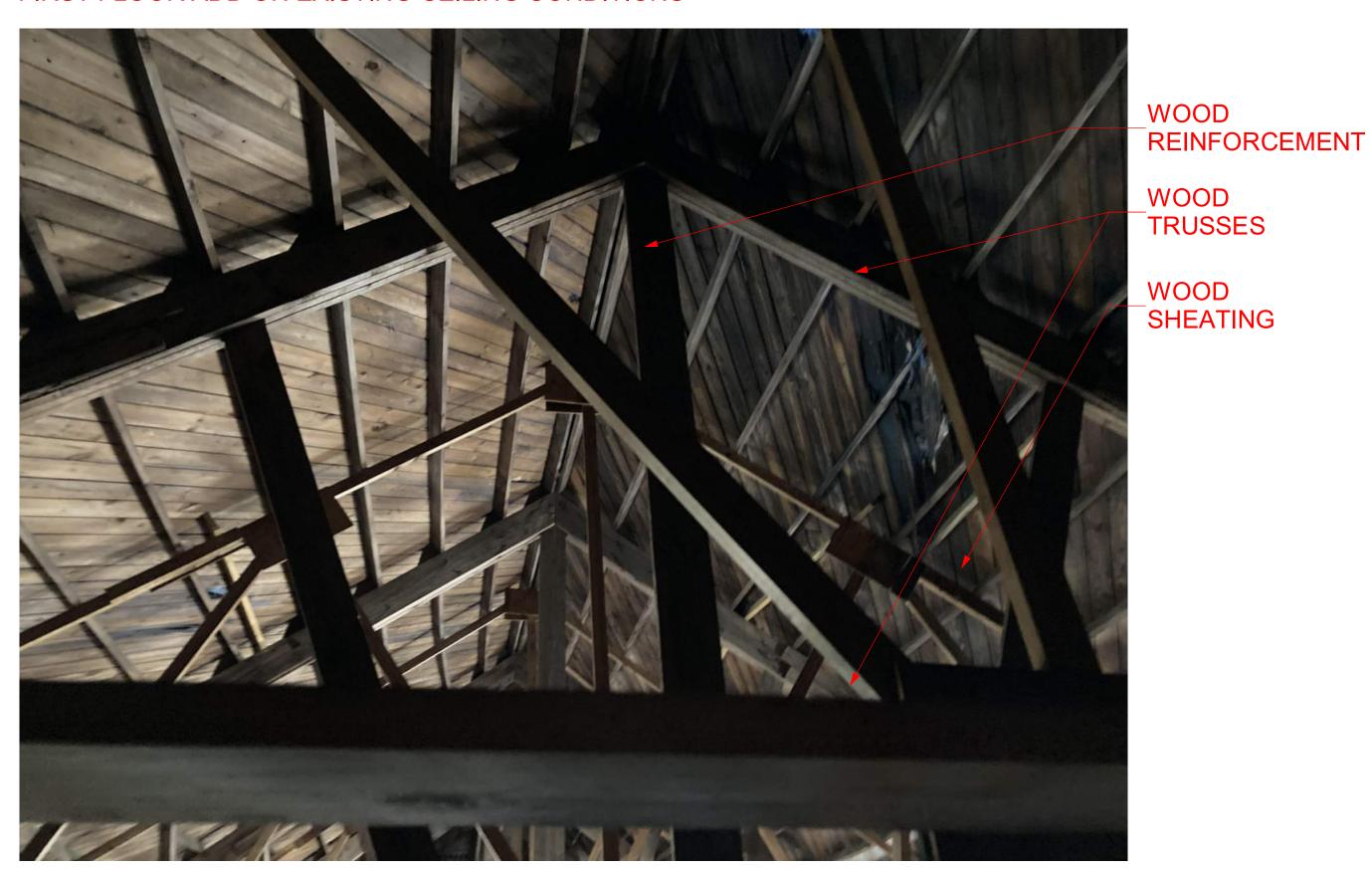
ROOF

STRUCTURE

WOOD SHEATING

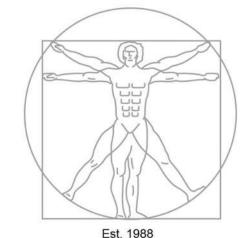
OVER RAFTERS

FIRST FLOOR ADD ON EXISTING CEILING CONDITIONS



ROOF WOOD TRUSSES STRUCTURE



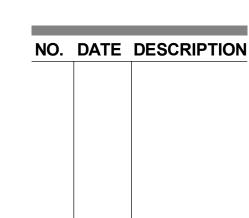


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DATE 10/24/2022 MODELED CHECKED MA/REG

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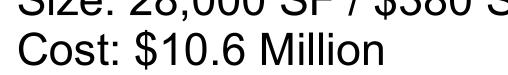
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CASE STUDY 01

CASE STUDY 02

BOYNTON BEACH ARTS & CULTURAL CENTER BOYNTON BEACH, FLORIDA

Size: 28,000 SF / \$380 SF















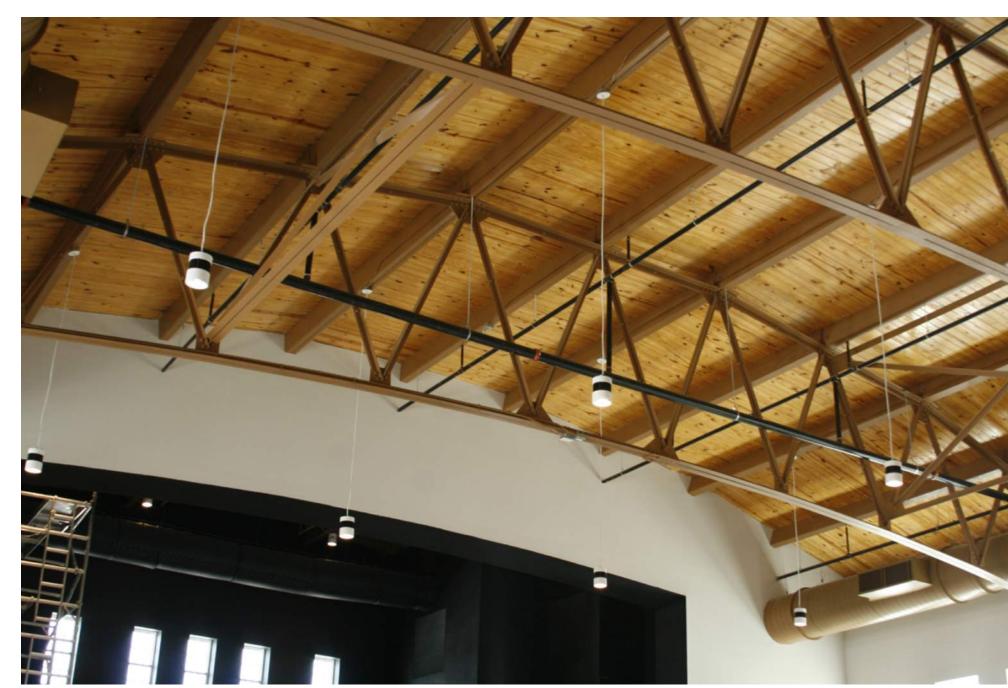


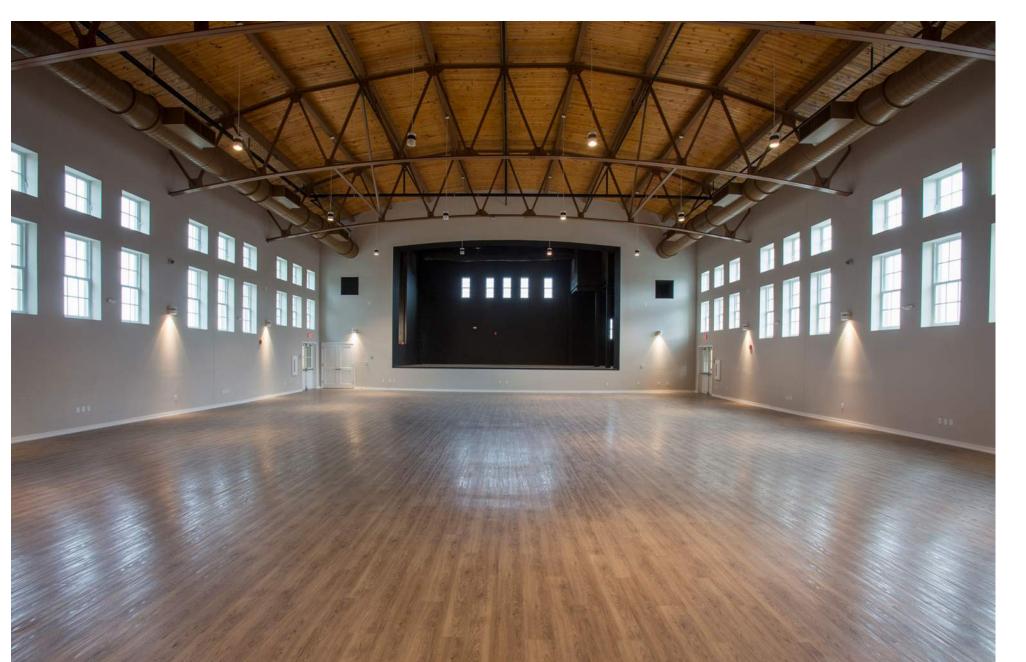




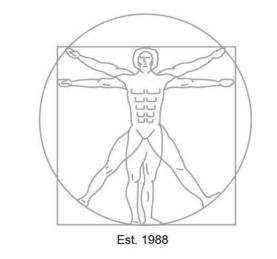








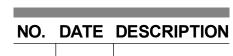




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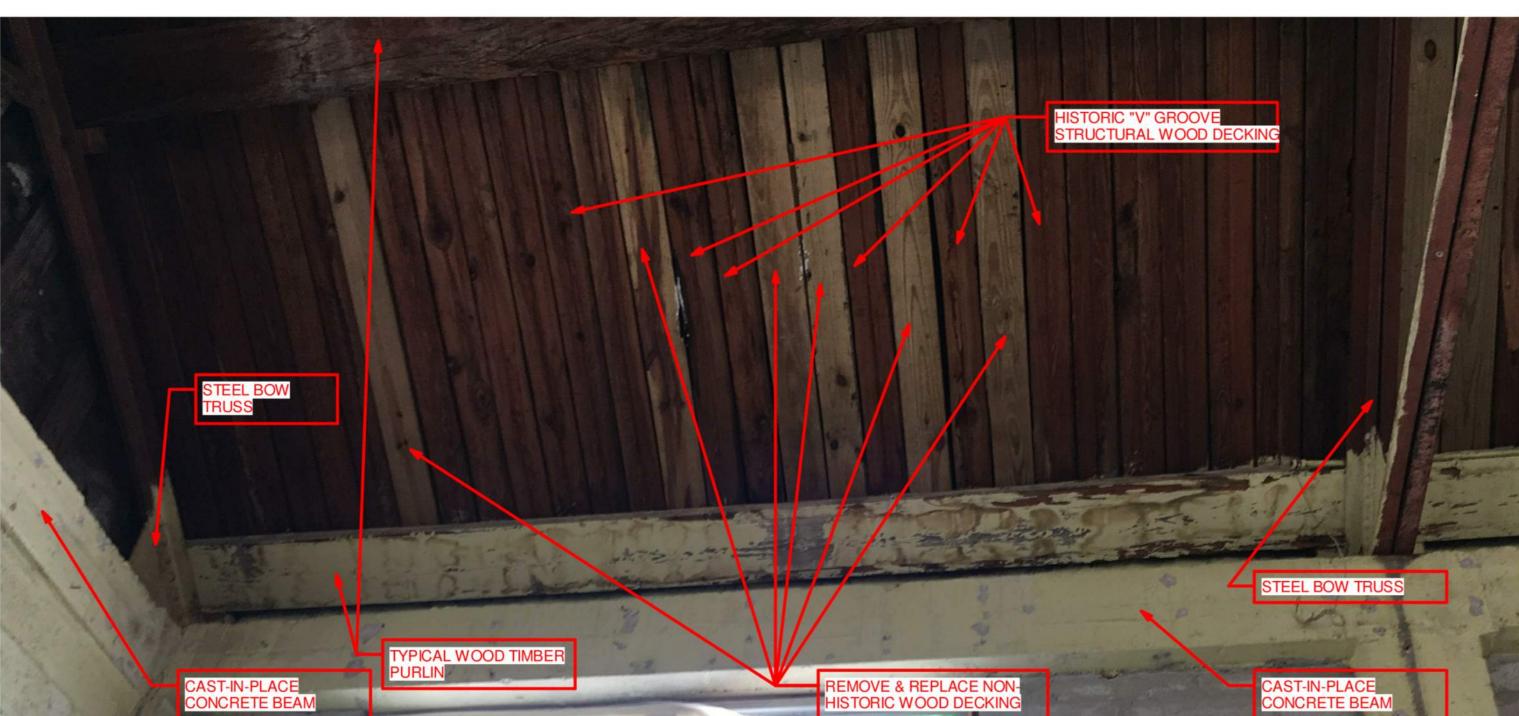
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CASE STUDY 02





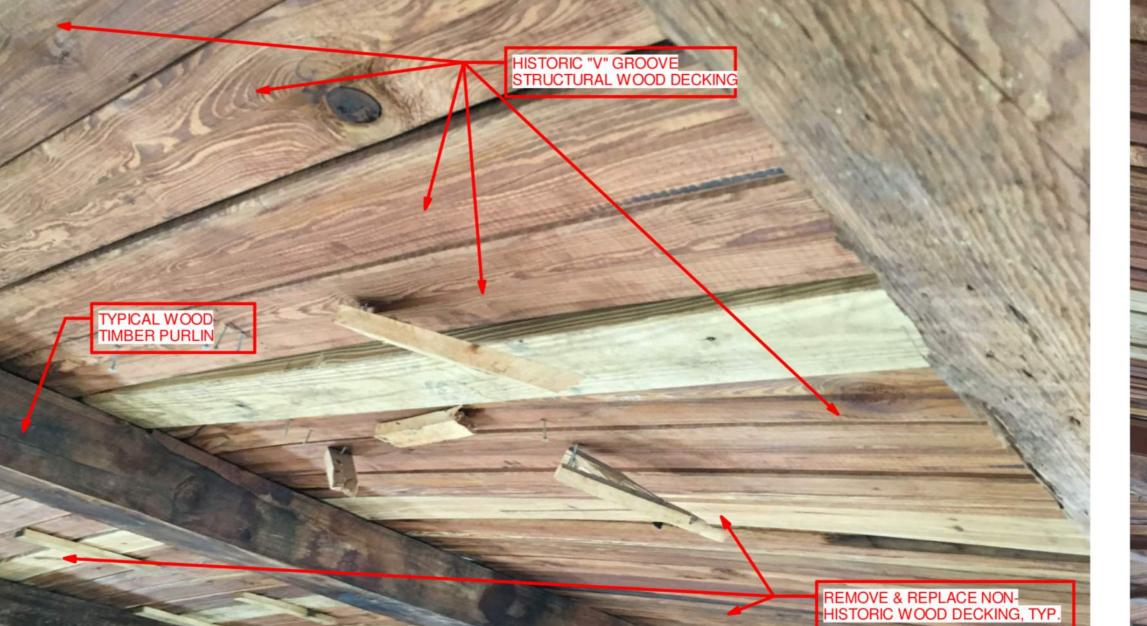


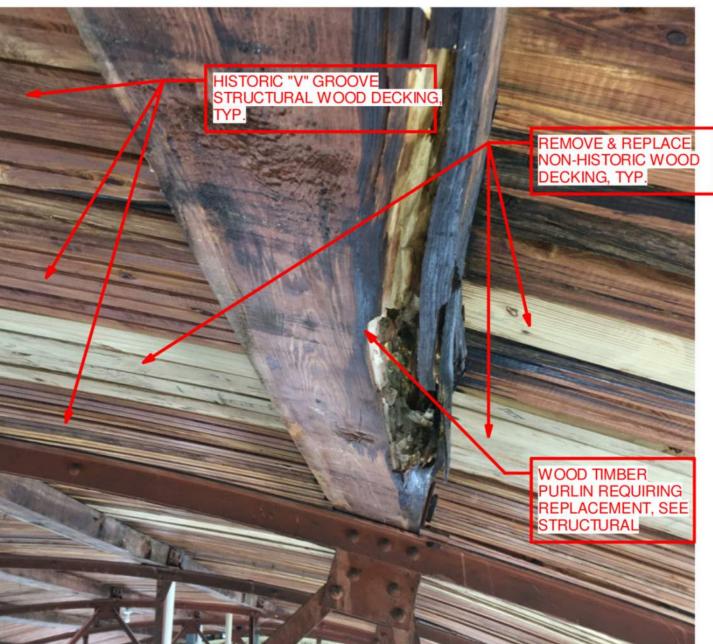


HISTORIC SUNSET LOUNGE Size: 20,500 SF / \$800 SF WEST PALM BEACH, FLORIDA Cost: \$16.4 Million

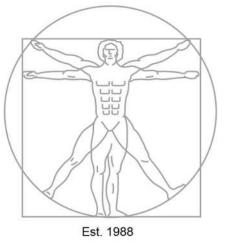












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CASE STUDY 03